

View of Centre and Willow Streets - Looking Southwest

Owner:	Acre Development Corporation Andover, MA
Developer/Project Manager:	Gary Martell Real Estate Equity Consulting Jamaica Plain, MA
Attorney:	Pulgini & Norton, LLP Braintree, MA
Architect:	RF Schmidt Architect, LLC, Brookline, MA



October 22, 2015

Mr. Brian Golden Director The Boston Redevelopment Authority One City Hall Square (9th Floor) Boston, MA 02201

RE: 1789 Centre Street, West Roxbury, MA Article 80E, Small Project Review Application Submission

Dear Director Golden:

On behalf of Acre Development Corporation and Gary C. Martell, I am pleased to submit this letter as Notice of our Small Project Review Application submission under Article 80E of the Boston Zoning Code, in connection with the proposed development at 1789 Centre Street, in the West Roxbury section of Boston.

The project site consists of an approximately 11,834 square foot lot bounded by Centre Street and Willow Street. Presently, the parcel contains the former used car business, known as West Roxbury Motors (the "Site), which is presently vacant, and considered an eyesore to the community. Redevelopment of the Site will enhance the street frontage of both Centre and Willow Streets, adding energy and vitality with the inclusion of sixteen (16) new residential condominiums, and (1) commercial condominium (the "Proposed Project").

The Proposed Project is a four-story building comprised of sixteen (16) residential units on floors two through four. Level one will consist of a resident's lobby, a commercial condominium, and storage and mechanical space. The development will provide twenty-nine (29) on-site parking spots, with approximately fifteen (15) contained in the interior of the structure and fourteen (14) exterior spaces.

The development team will be led by equity partner and project manager Gary C. Martell. Mr. Martell will lead a team of professional architects, engineers, contractors, and consultants with years of experience in the development of residential and commercial projects. The team has already hosted two neighborhood/abutters meetings sponsored with the cooperation of Chris Tracy form the BRA and Chris Rusk from the Mayor's Office of Neighborhood Services. In addition, there have been preliminary design meetings with BRA staff members and associated city agencies.

We intend to pursue the Article 80E Small Project Review Process for this Proposed Project and we look forward to continuing our strong working relationship with the BRA, the community, and the elected officials in the months ahead.

Very truly yours, John X. Pulgini

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Appendix A: Drawings:

- Cover Sheets (Renderings)
- Proposed Site Plan
- Landscaping Plan
- Architectural Drawings (A.1-A.7)

Appendix B: Howard Stein Hudson Transportation Study

Project Team

Principal Owner:	Acre Development Corporation 9 Prides Circle Andover, MA 01810
Developer/Project Manager:	Gary Martell Real Estate Equity Consulting 15 Brownson Terrace Jamaica Plain, MA 02130
Attorney:	Pulgini & Norton, LLP 10 Forbes Road West, Suite 410 Braintree, MA 02184
Architect:	RF Schmidt Architect, LLC 78 Wolcott Road Brookline, MA 02467
Landscape Architect:	Blair Hines Design Associates Landscape Architects 318 Harvard Street, Suite 25 Brookline, MA 02446
Civil Engineer:	Norwood Engineering Co., Inc. 1410 Route One Norwood, MA 02062

Investment Members:

Principal owner, Greg Alexandris has been developing and building residential and commercial projects in and around Greater Boston since 1987. Greg is a hands-on owner, involved in all aspects of the project, from design inception through construction and resident occupancy. Greg received his bachelor of Civil Engineering from the University of Lowell, Lowell, MA. *Please see a more detailed bio at the end of this section*.

Project Manager Gary C. Martell, a life-long Parkway Area resident, is well known in both the neighborhoods and city hall for investments, design, and project management with new construction and rehab developments through-out West Roxbury, Roslindale, Jamaica Plain, Roxbury, and Hyde Park. He is a lifelong resident of Boston and his track-record dates back to starting his first new construction job in Roslindale in 1987.

Project examples



The following projects were completed by the investment team over the past 6-7 years.

Warren Avenue, Hyde Park ... single-family, new construction.



22 - 26 Hawthorne Street, Roxbury... 8-units, new construction



302 - 306 Allandale Road ... 3 single-families, new construction



337 - 345 Belgrade Avenue, Roslindale ... 16-units commercial/residential building, new construction



17 Park Lane, Jamaica Plain ... single-family, new construction



60 - 64 Pond Street, Jamaica Plain ... 3-unit conversion, restoration



8 - 12 Stedman Street, Jamaica Plain ... 3-units, new construction



194 - 196 Durnell Avenue, Roslindale ... 2-units, new construction



Willoughby Estates, Andover, MA ... nine executive homes, new construction



Nathan Frye House, 166 North Main Street, Andover, MA ... Mixed-use, 12,000 sf (residential, retail and commercial) restoration, expansion and adaptive reuse of an historic landmark



62 Centre Street, Brookline, MA ... 3-unit restoration, 9,000 sf



29 Chestnut Street, Andover, MA....commercial build-out, modernization



Lincoln Woods, Andover, MA....24-Residential townhouse units, new construction

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ACRE DEVELOPMENT CORPORATION

OBJECTIVE

To develop, build, or renovate by using fundamental architectural principals that will not only fulfill each particular need but will also stand the test of time.

PRESIDENT AND PRINCIPAL

Greg Alexandris

EDUCATION

Bachelor of Science degree in Civil Engineering 1987 University of Lowell Lowell MA 01854

PRINCIPAL HISTORY & EXPERIENCE

Greg Alexandris has been involved in all aspects of the construction and development business since 1987. From the beginning he has been involved in commercial and industrial construction. However during the past two decades he has been focused on residential development.

NOTABLE PROJECTS COMPLETED IN GREATER BOSTON AREA

- **166 North Main Street Andover. (Nathan Frye Building)** Rehabilitation and modernization of historic mixed use building.
- Lincoln Woods. Development and construction of 24 Townhouses in Andover MA
- 28 Chestnut Street, Andover MA Commercial build out and Modernization
- Willoughby Estates Andover MA Construction of 9 Single Family Executive Residences

PROJECTS COMPLETED IN BOSTON AREA

- **250 Allston Street Brighton MA** three luxury townhouses built in 1998
- **254** Allston Street Brighton MA three luxury townhouses built in 1998
- **258 Allston Street Brighton MA** three luxury townhouses built in 2000
- **305 Summit Avenue** total gut rehab of five two bedroom units completed in 2001
- **62 Centre Street Brookline MA** total gut rehab of three luxury units completed in 2009

Neighborhood/Context



Overview aerial photo of proposed project site (former West Roxbury Motors).



Close-up aerial photo of proposed project site (former West Roxbury Motors).

Neighborhood/Context



View of proposed project site looking northeast at corner of Centre St. and Willow St.



View of proposed project site looking southwest at corner of Centre St. and Willow St.

Neighborhood/Context



View of proposed project site looking south on Willow St.



View of proposed project site looking south on Willow St. towards the entrance to Roche Bros.

Project Description

This new transit-oriented construction will consist of a mixed-use, four-story "for-sale" residential condominium building comprised of 16 dwelling units and 1 commercial condominium.

The first floor is comprised of a residents' lobby, commercial office space, accessory parking (for building occupants) and mechanical/electrical/utility space.

There is small penthouse to accommodate utilities and to provide access to a common roof deck.

The building does <u>not</u> have a basement.

The building will have an elevator.

The building will be constructed with a concrete foundation and a concrete slab on grade,

The building structure will be comprised of load-bearing, wood framed interior and exterior walls, with an engineered wood framed floor and roof.

The exterior walls will be clad with cement fiber siding.

Building Area:		
First Floor:	2,236 sf	
Second Floor:	6,952 sf (ea.)	
Third Floor:	6,772 sf (ea.)	
Fourth Floor:	6,872 sf	
Roof:	499 sf	
Total Area:	23,331 sf (does not include garage)	

Lot Size: 11,834 sf

Second through Fourth Floors - Residences:

4 - Two Bed Room/Two Bath Duplexes with gross areas ranging from 1,181 sf to 1,252 sf.

12 - Two Bed Room Units/Two Bath Flats with gross areas ranging from 1,070 sf to 1,342 sf.

Roof:

There is a 910 sf +/- common deck.

Design and Design Process

Design. The project proposes to demolish a former used car business, known as West Roxbury Motors.

The project scope includes remediation of the site and the transformation this eye-sore into 16-handsome new residences.

The building is sited to enhance the street frontage of both Centre and Willow Streets.

Site vehicular access and egress will be via a new curb cut on Willow Street. Four existing curb cuts (2 on Willow Street and 2 on Centre Street) will be eliminated. The proposed curb cut changes will improve pedestrian safety, allow for additional on-street parking.

The majority of on-site parking will be located within an enclosed garage. On-grade parking will be screened and landscaped to minimize public views of the parking.

The building is sited to complement the pattern of the existing buildings sited on the northerly end of the block. The free-standing West Roxbury Motors building, set far back from the street, will be replaced by a new residential building of contemporary design, sited to reinforce the position of street facing building façades on this block. In fact, the building plan bends in response to the curve of Centre Street.

The façade design and scale reflects the residential nature of the building through the development of window patterns and the materials used to clad the building. The mass of the building is broken down to a human-scale through plane changes, the articulation of bays and through use of architectural elements such as cornices and parapets which will create variations in the way the building meets the sky.

The main building entrances are proposed at the base of the building along Centre Street to enhance pedestrian activity and encourage street life. A generous commercial space fronts directly onto Centre Street, as does the main entrance for building residents. The base of the building is delineated with generous and welcoming storefront openings to further enhance these objectives. The substantial storefront openings activate the base of the building. The location of the main entrance to the building is well defined and the lobby has Willow Street facing windows. The small portion of the Centre Street façade which encloses a portion of the parking structure is designed to complement the commercial façades.

Head houses required to access the roof deck are sloped to minimize visibility. The varied roof line will minimize views of mechanical equipment in addition to creating a visual break at the top of the façade.

Design Process. The design of the proposed building has been shaped by substantial input from neighbors, the Mayor's Office and the BRA. During the preliminary design phase, the project team attended community meetings on June 15 and June 29, 2015 and preliminary design meetings with BRA and representative from the Mayor's Office.

As a result of these meetings the following issues were issues raised and addressed:

Public Realm:

- We were encouraged to design the Centre Street sidewalk in compliance with the City of Boston Complete Streets Guidelines.
 - The proposed design reflects the City of Boston Complete Streets Guidelines for a Neighborhood Connector Street.
- The owner was encouraged to take ownership of the City-owned property at the corner of Centre and Willow Streets.
 - The project team is in the process of obtaining the property from the City of Boston.

Traffic safety and parking:

- Many of the discussions with neighbors focused on traffic safety and parking issues. As a result of neighborhood and City of Boston Transportation Department Meetings:
 - A traffic study was commissioned. See attached exhibit C.

Site Plan and Building Location:

Discussions with the neighborhood and BRA resulted in this proposal for 16-dwelling units, 1-commercial condominium and 29 parking spaces.



The discussions also generated the exploration of several design options before settling on this scheme. The issues discussed and studied included:

- The siting and alignment of the building was studied and refined to reflect the curve of Centre Street
- A program proposed with 18-dwelling units:
 - Neighbors and the BRA requested fewer units
 - The project was revised to the current 16 units
 - Neighbors and the BRA requested more commercial storefront
 - The first floor plan and program was revised to maximize the opportunity for an active storefront on Centre Street
- The issues of traffic and parking were discussed;
 - Howard Stein Hudson was hired to prepare a transportation study

Architecture and Massing:

Discussions with the neighborhood and BRA resulted in this proposal for 16-dwelling units with 29 parking spaces.



Corner view of Centre and Willow Streets

The discussions also generated the exploration of several design options before settling on this scheme. The issues discussed and the options studied included:

- Creating an emphasis on the corner of Centre and Willow Streets
- Creating a masonry base for the building
- Cornices and projecting bays are used to:
 - Create human-scaled design elements
- The proposed building materials are:
 - Masonry veneer on the Centre Street and Willow Street first floor façades, with cementitious clapboard siding, aka Hardi-Siding on the remainder of the façades.
 - Complementary material colors will be selected.
 - The Hardi-Siding will be installed in varying exposures to help reduce the scale of the building and create a variety of façade details.

Landscape Design



The landscape plan for the project at 1789 Centre Street is proposed as a durable and beautiful complement to the architecture in the center of the commercial district of West Roxbury. The redevelopment of the lot will include a planting palette of six new street trees and sixty-four new shrubs, perennials, and vines. The proposed plan will enliven and provide comfort for the shopping streetscape by adding some much needed shade to the public sidewalk, where possible. The street trees are proposed to be planted within enlarged sand-based planting medium areas extending below the new concrete paving for ample root zone growth with tree grates to maximize the accessible sidewalk.

Providing proper screening and separation from the neighbors has been carefully considered. Between the parking area and the sidewalk along Willow Street, 6' height lattice fencing will provide screening of the parking while still allowing some visibility and safety to the residents accessing their vehicles. Between the abutters to the north and the project site will be a 6' height wood board fence, which will also screen the parking. Along the property border between the project site and their neighbors on Centre Street, a combination of lattice fence with vines and wood board fence will provide screening of the parking. Along the western edge of the property, the access to the garage will feature pervious concrete pavers with a 6" height granite curb providing protection from vehicles exiting the neighboring property.

The streetscape along Centre Street will continue the design standards established elsewhere for this corridor. A 2' wide brick accent edge along the public sidewalk will be installed where possible. A special saw cut concrete paving pattern will bring a subtle distinction to the main entrances for the residences and commercial space. A bike rack along the Centre Street corridor is also proposed on the plan, which will support multi-modal transit in the area. Framing the entrance to the rear parking, will be four fastigiated sweetgum trees in tree grates and sand-based planting medium area.

Zoning Review

Article 56 West Roxbury Neighborhood District. Zoning District: NS – Neighborhood Shopping Subdistrict per Map 11B.

The project zoning was reviewed and approved by the City of Boston Inspectional Services, Planning and Zoning Division.

Anticipated Permit Requirements

The table below lists the permits and approvals that are anticipated for this project.

Agency	Approval
Boston Redevelopment Authority	Article 80 Small Project Review (SPR)
Inspectional Services	Compliance with MA State Building Code for issuance of building permit
City of Boston Public Improvements	Sidewalk and intersection design
Commission	

Construction Impact

No negative impact is anticipated on the site or surrounding area. All staging, construction materials, equipment, storage, and most worker/parking can be accommodated on the site. We anticipate obtaining a permit (from the public works department) to install temporary fencing on parts of the public sidewalk.

In addition, there will be numerous police details anticipated as necessary throughout the construction process. The biggest impact will be at the excavation/foundation stage of the job because of limited storage and staging for material and equipment. All efforts will be made to minimize the foundation installation impact on the traffic and pedestrian flows. The same attention to minimize any negative impact to the area will be followed by the project management team for the entire length of construction.

Traffic, Parking and Site Access

This transit-oriented site is located within 0.25 of a mile of Commuter Rail and 3 miles of the Forest Hills T Station on the Orange Line. The no. 35, 36 and 37 bus stops are adjacent to and directly across the street from the site. These bus lines provide riders with access to the Orange Line.



Resident parking is provided on site in excess of the rate required by the City of Boston Zoning Code (1.5 spaces per dwelling unit) at a rate of 1.53 spaces per dwelling unit.

Based on the evaluation presented in the transportation study, the Project at 1789 Centre Street will have minimal impact on the surrounding transportation infrastructure. The Project is expected to generate ten or fewer trips per hour during the peak commuter periods. This is the equivalent of one additional vehicle every six minutes, which is within the range of typical fluctuations in daily and hourly traffic volumes. The surrounding transportation infrastructure has the capacity to accommodate the minimal amount of additional trips expected to be generated by the Project without the need for any additional improvements.

The access to the site was studied in and the Willow Street driveway was determined to be most desirable from a safety and traffic operations perspective.

For a more in-depth analysis of traffic and parking see Exhibit C.

Proposed New Mixed-Use,16-Dwelling Unit Residential Condominium Building 1789 Centre Street, West Roxbury, MA

Article 80 Small Project Review Submission: October 21, 2015



List of Drawings

CS.1	Cover Sheet Site Plan
A.1 A.2	Landscaping First Floor Pla Second Floor
A.3	Third Floor Pl
A.4	Fourth Floor I
A 5	Roof Plan
A.6	Exterior Eleva
A.7	Building Sect

Building Area:
Floor 4: Floor 3:
Floor 2: First Floor:
Total Area:

rf schmidt ARCHITECTS

78 Wolcott Road Chestnut Hill, MA 02467.3109

617 731 7770



ations ion

Project Description: New four-story Mixed-Use, Residential Condominium Building comprised of 16 dwelling units. The first floor is comprised of a lobby, commercial office space, accessory parking. (for building occupants) and mechanical/electrical/utility space. There is small penthouse to accommodate the elevator head override and to provide access to a common roof deck. The building does not have a basement. The building will be constructed with a concrete foundation, load-bearing, wood framed interior and exterior walls, with an engineered wood framed floor and roof for the majority of the residences. The first floor will be constructed with a steel frame and concrete slab on grade and a concrete slab on metal deck at the second floor. The exterior walls will be clad with cement fiber siding. The building will have an elevator.

sf (does not include garage)





ZONING CLASSIFICATION: WEST ROXBURY NEIGHBORHOOD DISTRICT NEIGHBORHOOD SHOPPING SUBDISTRICT (NS) ARTICLE 56 - TABLES B, F & I - MAP No. 11B

DIMENSIONAL REQUIREMENTS

USE: 16 RES. & 1 COMM. UNIT (A)	REQUIRED	PROPOSED
MINIMUM LOT SIZE	NONE	11,834 sf (B)
MAYIMUM FLOOR AREA RATIO	2.0	1.971
MAYIMUM GROSS FLOOR AREA	23,688 SF	23.331 SF (C)
MAXIMUM BUILDING HEIGHT	35 FT	34.71 FT (H)
MIN. LOT AREA/DWELLING UNIT	N/A	N/A
MIN. OPEN SPACE/DWELLING UNIT	800 SF (1)	910 SF (D)
MINIMUM LOT WIDTH	NONE	100.80 FT
MINIMUM LOT FRONTAGE	NONE	103.01 FT (E)
MINIMUM FRONT YARD	NONE (2)	0.20 FT
MINIMUM SIDE YARD (3)	NONE	0.54 FT (F)
MINIMUM DEAD YARD (4)	40 FT	40.66 FT/36.83 FT (F
PARKING SPACES	26 SPACES	29 SPACES (G)

ZONING NOTES

A. ALLOWED USE - SEE SECTION 56-45 TABLE 'B' - A MULTI-FAMILY DWELLING IS ALLOWED (ABOVE THE FIRST FLOOR PER FOOTNOTE 7). ALLOWED ACCESSORY USES INCLUDE GENERAL OFFICE SPACE ALONG WITH PARKING AND ACCESSORY SERVICES FOR THE BUILDING RESIDENTS.

B. THE PROPOSED LOT AREA INCLUDES A 300 SQUARE FOOT PORTION OF THE STREET LAYOUT AT THE INTERSECTION OF WILLOW STREET AND CENTRE STREET AS SHOWN ON THIS PLAN.

THE PROPOSED GROSS FLOOR AREA (SHEET A.1) IS SHOWN ON THE ATTACHED ARCHITECTURAL PLANS PREPARED BY, RF ARCHITECT, LLC.

D. THE PROPOSED 910 SQUARE FOOT OPEN SPACE AREA ON A COMMON ROOF DECK IS SHOWN ON THE ATTACHED ARCHITECTURAL PLANS (SHEET A.5) PREPARED BY, RF SCHMIDT, ARCHITECT, LLC.

E. LOT FRONTAGE IS ON CENTRE STREET. SEE SECTION 56-36.1, STREET WALL CONTINUITY SHALL APPLY TO ANY PROPOSED PROJECT, EXCEPT A PROPOSED PROJECT FOR RESIDETIAL USE ... SECTION 56-40.2 TRAFFIC VISIBILITY ACROSS CORNER "WHENEVER A MINIMUM FRONT YARD IS REQUIRED AND THE LOT IS A CORNER LOT ..." (NOT APPLICABLE, THERE IS NO MINIMUM FRONT YARD IN THE NEIGHBORHOOD SHOPPING SUBDISTRICT).

F. MAIN BUILDING/BUILDING PROJECTION SETBACKS SEE ARTICLE 2A - SIDE YARD/REAR YARD DEFINITIONS - ".... NO STRUCTURE SHALL BE ERECTED EXCEPT (D) OTHER FIRE ESCAPES, BAYS, BALCOMES, CHINNEYS AND FLUES NO COMING WITHIN THREE (3) FEET OF ANY SIDE LOT LINE AND NOT EXTENDING INTO THE SIDE YARD MORE THAN ONE-THIRD (1/3) OF THE WIDTH OF SUCH YARD NOR MORE THAN THREE AND ONE-THIRD (3-1/3) FEET IN ANY EVENT...."

G. PARKING REQUIREMENTS FROM TABLE I RESIDENTIAL PARKING – 1.5 SPACES/DWELLING UNIT – 1.5x16 UNITS = 24 SPACES REQUIRED OFFICE / RETAIL AREA 950 SF± – 2 SPACES PER 1000 SQUARE FEET – <u>2 SPACES REQUIRED</u> REQUIRED PARKING 26 SPACES – PROPOSED 29 SPACES (15 ♥ 8.5¹x20¹ & 14 ♥ 7¹x18¹)

H. SEE AVERAGE GRADE AT BUILDING AND BUILDING HEIGHT CALCULATIONS BELOW.

WEST ROXBURY NEIGHBORHOOD DISTRICT — TABLE F — FOOTNOTES 1. IN A NEIGHBORHOOD BUSINESS SUBDISTRICT, ALL OR A PORTION OF REQUIRED USABLE OPEN SPACE MAY BE MET BY SUITABLY DESIGNED AND ACCESSIBLE SPACE ON BALCOMES OF MAIN BUILDINGS OR ON ROOFS OF WINGS OF MAIN BUILDINGS, OR ON THE ROOFS OF ACCESSORY BUILDINGS. REQUIRED DEFN SPACE IS SO SF/UNIT (16 UNITS \times 50 SF = 800 SF OF OPEN SPACE IS REQUIRED)

2. SEE SECTION 56-36.1, STREET WALL CONTINUITY SHALL APPLY TO ANY PROPOSED PROJECT, EXCEPT A PROPOSED PROJECT FOR RESIDETIAL USE

3. IN A NEIGHBORHOOD BUSINESS SUBDISTRICT, NO SIDE YARD IS REQUIRED EXCEPT IN THE CASE OF A LOT WITH A SIDE LOT LINE ABUITING A RESIDENTIAL SUBDISTRICT, WHICH SHALL HAVE SIDE YARDS AS IF IT WERE IN SUCH ABUITING DISTRICT ... (SEE FOOTNOTE 6. EVERY SIDE YARD SO REQUIRED THAT DOES NOT ABUT A STREET LINE SHALL, ALONG EVERY LOT LINE ON WHICH SUCH YARD ABUTS, BE AT A LEVEL NO HIGHER THAN THAT OF THE LOWEST WINDOW SILL OF THE LOWEST ROOM DESIGNED FOR HUMAN OCCUPANCY OR SO OCCUPIED, AND RELYING UPON NATURAL LIGHT OR NATURAL VENTILATION FROM WINDOWS OPENING ON SUCH YARD.

4. IN A NEICHBORHOOD BUSINESS SUBDISTRICT, EVERY REAR YARD REQUIRED BY THIS CODE THAT DOES NOT ABUT A STREET LINE SHALL ALONG EVERY LOT LINE ON WHICH SUCH YARD ABUTS, BE AT A LEVEL NO HIGHER THAN THE LEVEL OF THE LOWEST WINDOW SILL IN THE LOWEST FOOM DESIGNED FOR HUMAN OCCUPANCY OR SO OCCUPIED, AND RELIVING UPON NATURAL LIGHT OR NATURAL VENTILATION FROM WINDOWS OPENING ON SUCH YARD. (SEE ARTICLE 2A SIDE YARD (REAR YARD) PROJECTIONS DEFINITON ABOVE)

DESIGN REVIEW AND DESIGN GUIDELINES 1. ARTICLE 80-E2.1(b)(1) - SMALL PROJECT REVIEW - REQUIRED FOR PROPOSED PROJECTS WITH OVER 20,000 SF AND LESS THAN 50,000 SF

II. ARTICLE 80-E2.1(b)(II) - SMALL PROJECT REVIEW - REQUIRED FOR PROPOSED PROJECTS WITH 15 NEW DWELLING UNITS OR MORE

III. SECTION 56-35.2(c) - PARKING SCREENING - THE PROJECT WAS REVIEWED WITH THE BRA AND WILL BE ADEQUATELY SCREENED WITH PLANTINGS AND FENCING. A LANDSCAPING PLAN WILL BE PREARED INCORPORATING 'BRA' COMMENTS AND WILL BE SUBMITTED WITH THE FORMAL ARTICLE 80E SMALL PROJECT REVIEW.

IV. SECTION 56-35.2 (h) CONTEMPORARY DESIGN - THE LOCATION AND CONTEMPORARY DESIGN OF THE BUILDING, INCLUDING THE DESIGN AND LOCATION OF BAYS AND BUILDING ENTRANCES REDUCE THE BULK OF THE BUILDING WILL BE REVIEWED BY THE 'BRA' WHO WILL PROVIDE FINAL DESIGN COMMENTS AS PART OF THE FINAL DESIGN REVIEW.

GRADE AT	PERIMETER OF	BUILDING
GRADE	LENGTH (FT)	PRODUCT
154.25	8.63	1331.18
151.75	5.17	784.55
154.25	32.26	4976.11
151.25	5.17	781.96
154.25	19.17	2956.97
151.10	5.17	781.19
154.25	20.41	3184.24
152.12	38.34	5832.28
154.25	44.52	6867.21
	178.84	27495.69

PRODUCT / PERIMETER = AVERAGE GRADE 27,495.69 / 178.84 = 153.74 AVERAGE GRADE ELEVATION = 153.74±

AVEARGE GRADE AT BUILDING ALONG STREETS = 153.74 AVERAGE GRADE AT PERIMETER OF BUILDING = 153.26 AVERAGE GRADE AT WILLOW STREET LINE = 151.1 \pm AVERAGE GRADE AT CENTRE STREET LINE = 151.1 \pm DIFFERENCE 153.74 -151.1 = 2.64 FT < 5.00 FT

*ARTICLE 2A GRADE DEFINITION "THE AVERAGE ELEVATION AT THE STREET CAN BE NO MORE THAN FIVE FEET BELOW AVERAGE ELEVATION OF GROUND INMEDIATELY CONTIGUOUS TO THE BUILDING"



ROOF BEAM ELEVATION (+8.94)* 4TH FLOOR ELEVATION (+8.94)* 3RD FLOOR ELEVATION (+8.94)* 2ND FLOOR ELEVATION (+9.85)* AVG GRADE AT BLDG (ALONG STREETS) 187.92 178.98 170.04 161.10 153.74 153.26 AVG GRADE AT BLDG (PERIMETER) 1ST FLOOR ELEVATION (COMM. UNIT) 151.25 . INFORMATION FROM ARCHITECTURAL PLANS

MAXIMUM BUILDING HEIGHT ALLOWED IS 35 FEET ROOF ELEVATION - AVERAGE GRADE = BUILDING HEIGHT 187.92 FEET - 153.74 FEET = 34.18 FEET PROPOSED BUILDING HEIGHT = 34.18 FEET < 35 FEET

BUILDING PERMIT PLAN 1789 CENTRE STREET BOSTON, MASS. (WEST ROXBURY - 02132-1941) SCALE: 1"=10' SEPTEMBER 3, 2015 NORWOOD ENGINEERING CO., INC. CIVIL ENGINEERS & LAND SURVEYORS 1410 ROUTE ONE, NORWOOD, MA 02062 PHONE: 781-762-0143 FAX 781-762-8595 2.5 10 METERS 0 2.5 5 FEET 0 5 10 30 SHEET No. 1 OF 1 8291-14

PROPOSED PLANT LIST

KEY	QTY	LATIN NAME	COMMON NAME	SIZE	NOTES
TREE	S				
GT	2	Gledeitsia triacanthos var. inermis	Common Thornless Honeylocust	2.5"-3" cal.	B&B
LS	4	Lirodendron stryraciflua 'Slender Silhouette'	Slender Silhouette Sweetgum	2.5"-3" cal.	B&B
SHRU	JBS/GR/	ASSES/VINES			
CA	5	Calamagrostis × acutiflora 'Karl Foerster'	Feather Reed Grass	#2 Pot	
CV	6	Clematis virginiana	Virgin Bower's Vine	#2 Pot	
FD	8	Fargesia sp. Dracocephala'	Dragon's Head Clumping Bamboo	#2 Pot	
IC	8	llex crenata 'Dwarf Pagoda'	Dwarf Japanese Holly	#3 Pot	
PA	13	Penniseum alopecuroides 'Little Bunny'	Dwarf Fountain Grass	#I Pot	

PERE	NNIA	LS			
AM	8	Achillea millefolium 'Apple Blossom'	Yarrow	2 gal.	Pots
GR	7	Geranium rozanne	Cranesbill	2 gal.	Pots
HE	3	Heuchera 'Encore'	Coralbells	2 gal.	Pots
NF	6	Nepeta faassenii	Catmint	2 gal.	Pots

PLANTING NOTES

- I. All plant material shall be approved by the Landscape Architect prior to arrival on the site.
- 2 All plant material shall conform to the guidelines established by "The American Standard for Nursery Stock", published by the American Association of Nurserymen, Inc.
- 3. No substitution of plant species will be allowed without the approval of the Landscape Architect.
- 4. The Contractor shall locate and verify all utility line locations prior to staking and report any conflicts to the Landscape Architect
- All plants shall be staked out in their approximate location by the Contractor. The Contractor shall adjust the locations
 of these stakes as required by the Landscape Architect to account for subsurface utilities, other field conditions and to achieve design intent. Final locations must be approved by the Landscape Architect prior to planting.
- 6. No planting shall be installed before acceptance of rough grading of topsoil.
- The rootballs of trees shall be planted 3" above adjacent finished grade. Excavate holes no deeper than the rootball of trees. Holes shall be at least 3' greater in diameter than root ball. Backfill planting hole with 'planting mix'. All plants 7. which settle out of plumb or below finished grade shall be immediately replanted.
- 8. The rootballs of shrubs shall be planted 2" above adjacent finished grade. Excavate holes no deeper than the rootball of shrubs.
- 9. All shrubs, groundcovers and perennials shall be planted in continuous planting beds. All beds shall be excavated 12" and the topsoil and subsoil set aside for reuse. Remove all stone and debris from excavated soil. Backfill beds with 12" of 'planting mix' before planting shrubs, perennials and groundcovers.
- 10. 'Planting Mix' shall consist of 2 parts of topsoil saved from site excavations and 1 part compost. Thoroughly mix to create uniform blended mixture. If insufficient topsoil is available on the site, mix existing soil in a ratio of 1 part soil to 1 part compost. Remove all stones and debris larger than 2" from planting mix.
- 11. All beds as shown on the drawings shall be edged with a 4" trench neatly cut and backfilled with bark mulch. All beds shall be covered with no less than 2"depth settled bark mulch and no greater than 3" depth bark mulch.
- 12. All plants are to be thoroughly watered after installation, at least twice within the first 24 hours.



Concrete with Special Saw Cut Pattern









Blair Hines Design Associates LANDSCAPE ARCHITECTS I 1789 Centre Street West Roxbury, MA Illustrative Landscape Plan



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PARKING CALCS: 56-TABLE - I

26 TOTAL REQUIRED

29 PROPOSED

1.5 SPACES per D.U.= 24 2.0/1,000 SF OFFICE/RETAIL= 2

PROGRAM: Roof = 499 sf 4th fl=6,872 sf 3rd fl=6,772 sf 2nd fl=6,952 sf

Total: 17 Units 16 Residential units (12 - 2BR flats + 4 2BR duplexes) 1 Commercial

1st Floor = 2,236 sf

2 Total Area = 23,331 sf

SITE: LOT SIZE: 11,834sf FAR=2.0/23,668 sf ALLOWED FAR=1.97 PROPOSED



NEM MIXED-USE BUILDING:

1789 CENTRE STREET WEST ROXBURY, MA

4			
	Scale	ſ	Commission No.
	0 4 8	16	1405
	Date		Issue
	3 Sep 15		Zoning Submission
	21 Oct 15		Art. 80 Small Project Review
	@2015 RF Schmidt	Archite	NOT, LLG
	Title		

Second Floor Plan

A.2





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Third Floor Plan

A.3



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PARKING CALCS: 56-TABLE - I

29 TOTAL REQUIRED

29 PROPOSED

1.5 SPACES per D.U.=27 2.0/1,000 SF OFFICE/RETAIL=2

PROGRAM: Roof = 511 sf 2nd-4th Floors = 7,075 sf 2 - 1BR/1B 4 - 2BR/2B

Total: 19 Units 18 Residential units (12 - 2BR + 6 - 1BR) 1 Commercial

1st Floor = 1,691 sf 2 Total Area = 23,487 sf

> SITE: LOT SIZE: 11,534sf + 375sf=11,909sf FAR=2.0/23,818 sf ALLOWED FAR=1.97 PROPOSED

> > rf schmidt

78 Wolcott Road Chestnut Hill, MA 02467,3109 617.731.7770



NEM MIXED-USE BUILDING:

1789 CENTRE STREET WEST ROXBURY, MA

4		
	Scale	Commission No.
	0 4 8 10	1405
	Date	Issue
	3 Sep 15	Zoning Submission
	21 Oct 15	Art. 80 Small Project Review
	G2015 RF Schmidt Arch	nitest, LLG

A.4



Roof Plan A.5



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A 1 Notes SEE A.7 FOR BUILDING HEIGHT

в





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TECHNICAL MEMORANDUM



To:	Mr. Gary Martell	DATE:	July 23, 2015
FROM:	Michael A. Santos, P.E., PTOE Michael Littman	HSH PROJECT NO .:	2015089.00
SUBJECT:	1789 Centre Street Transportation Evaluation		

Introduction

Howard Stein Hudson (HSH) has conducted an evaluation of the transportation impacts of the redevelopment of 1789 Centre Street (the "Project") in West Roxbury, Massachusetts. The site currently contains the former West Roxbury Motors and will be redeveloped to include a total of 18 residential condominium units and one commercial space. The commercial space will be approximately 1,000 square feet (sf). The Project site is well situated to take advantage of numerous public transportation opportunities and car sharing services. The Massachusetts Bay Transportation Authority (MBTA) operates three bus lines that travel past the site (bus routes 35, 36, and 37), providing access to Roslindale Village and Forest Hills Station in Jamaica Plain. Highland Station and Bellevue Station are also within walking distance of the Project site and serve the MBTA's commuter rail Needham Line, providing convenient access into South Station and downtown Boston.

This study includes an evaluation of existing conditions and future conditions with and without the Project. Based on the results of the evaluation summarized in this memorandum, the Project is expected to have negligible impact on the surrounding transportation infrastructure.

Project Description

The Project site is located at 1789 Centre Street in Boston's West Roxbury neighborhood. The site is bounded by Centre Street to the south, Willow Street to the east, and residential properties to the north and west. The Project site currently contains the former West Roxbury Motors and has two curb cuts along Centre Street and one curb cut along Willow Street.

The Project includes the demolition of the existing building and the construction of a new building containing 18 residential condominium units and approximately 1,000 sf of ground floor commercial space that will front Centre Street. A total of 29 parking spaces will be provided on the site (27 to serve the residential uses and two to serve the commercial space).

11 BEACON STREET, SUITE 1010 | BOSTON, MASSACHUSETTS 02108 | 617.482.7080



Vehicular access/egress will be provided by way of a driveway that will intersect Willow Street, in the approximate location of the existing curb cut. The existing curb cuts along Centre Street will be closed as part of the Project.

Study Area

The study area selected for the Project includes the intersection of Centre Street/Willow Street and a qualitative analysis of the surrounding roadway network including Centre Street, Willow Street, Maple Street, and Alhambra Road.

Study Methodology

This transportation study and supporting analyses were conducted in accordance with BTD guidelines and is described below.

The existing conditions analysis includes an inventory of the existing (2015) transportation conditions such as roadway capacities, traffic characteristics, parking and curb usage, transit, pedestrian circulation, bicycle facilities, and Site conditions. Existing vehicle, bicycle, and pedestrian counts were collected in June 2015. The traffic counts form the basis for the transportation analysis conducted as part of this evaluation.

The future transportation conditions analysis evaluates potential transportation impacts associated with the Project. Long-term impacts are evaluated for the year 2020, based on a five-year horizon from the existing year (2015). Expected roadway, parking, transit, pedestrian, bicycle accommodation, and loading capacities and deficiencies, if any, are identified. This section includes the following scenarios:

- The 2020 No-Build conditions scenario includes both general background traffic growth and traffic growth associated with specific developments that are planned in the vicinity of the Site. Transportation infrastructure improvements in the study area are identified and incorporated into the 2020 No-Build conditions.
- The 2020 Build conditions scenario includes Project-generated traffic volume estimates added to the traffic volumes developed as part of the 2020 No-Build conditions scenario.

The final part of the transportation study identifies measures to mitigate Project-related impacts and to address any traffic, pedestrian, bicycle, transit, safety, or construction related issues that are necessary to accommodate the Project. An evaluation of short-term traffic impacts associated with construction activities is also provided.

Existing Conditions

Existing Roadway Conditions

The study area roadways are described below. The descriptions reflect functional classifications by the Massachusetts Department of Transportation (MassDOT) Highway Division's Office of Transportation Planning.

Centre Street is a two-way, four lane roadway located adjacent to the south side of the Project site. Centre Street is classified as an urban minor arterial roadway under BTD jurisdiction that runs in an east-west direction in the immediate vicinity of the site and more generally in a north-south direction between Jackson Square in Jamaica Plain to the north and the Dedham Town Line to the south. On-street parking is prohibited on both sides of the roadway adjacent to the site; however, parking is provided along Centre Street to the east and west of the Project. Sidewalks are provided along both sides of the roadway.

Willow Street is a one-way southbound, one lane roadway located adjacent to the east of the Project site. Willow Street is classified as a local roadway under BTD jurisdiction and runs in a predominately north-south direction between Farmington Road near the VFW Parkway to the north and the Roche Brothers parking lot to the south. In the vicinity of the Project, unrestricted on-street parking is provided on the east side of the roadway and sidewalks are provided on both sides of the roadway.

Existing Intersection Conditions

The existing conditions at the study area intersection are described below.

Centre Street/Willow Street is a four legged, signalized intersection with four approaches. The Centre Street eastbound approach consists of two through travel lanes and an exclusive right-turn lane. The Centre Street westbound approach consists of an exclusive left-turn lane and two through travel lanes. The Willow Street northbound approach consists of two lanes, a left-turn only lane, and a right-turn only lane, with on-street parking and serves as the driveway to the Roche Brothers commercial plaza. The Willow Street southbound approach is one-way and consists of a single shared left-turn/thru/right-turn lane, with on-street parking provided along the east side of the roadway.



Concrete sidewalks and provided along both sides of all approaches. Crosswalks with handicap accessible ramps and count-down pedestrians signal indications are provided across all approaches to the intersection.

Existing Traffic Conditions

Traffic movement data was collected the study area intersection on Tuesday June 30, 2015. Manual turning movement counts (TMCs) and vehicle classification counts were conducted during the weekday a.m. and p.m. peak periods (7:00-9:00 a.m. and 4:00-6:00 p.m., respectively) for the study area intersection. The vehicle classification counts included car, truck, pedestrian, and bicycle movements and are presented in **Figure 1**. Based on the TMCs, the peak hours of vehicular traffic throughout the study area are 7:45-8:45 a.m. and 4:45-5:45 p.m. The detailed traffic counts are provided in the **Appendix**.



a.m. Peak Hour (7:45 a.m. - 8:45 a.m.) SITE ຄ∓ຊ|⊷472 ↓ ↓ • 64 ┫ CENTRE STREET 38 **→** 54 **→** 453→ 42→ WILLOW STREET p.m. Peak Hour (4:45 p.m. - 5:45 p.m.) SITE 33 25 27 ← 584 **↓** ↓ ↓ **- - 111** CENTRE STREET 505→ • 71 🔫 62 03 WILLOW STREET Not to scale.

Figure 1. Existing (2015) Condition Turning Movement Counts

Existing Traffic Operations

The criterion for evaluating traffic operations is level of service (LOS), which is determined by assessing average delay experienced by vehicles at intersections and along intersection approaches. Trafficware's Synchro (version 9) software package was used to calculate average delay and associated LOS at the study area intersections. This software is based on the traffic operational analysis methodology of the Transportation Research Board's 2000 Highway Capacity Manual (HCM).

LOS designations are based on average delay per vehicle for all vehicles entering an intersection. **Table 1** displays the intersection LOS criteria. LOS A indicates the most favorable condition, with minimum traffic delay, while LOS F represents the worst condition, with significant traffic delay. LOS D or better is typically considered desirable during the peak hours of traffic in urban and suburban settings.

Level of Service	Average Stopped Delay (seconds/vehicle)							
	Signalized Intersection	Unsignalized intersection						
А	≤10	≤10						
В	>10 and ≤20	>10 and ≤15						
С	>20 and ≤35	>15 and ≤25						
D	>35 and ≤55	>25 and ≤35						
E	>55 and ≤80	>35 and ≤50						
F	>80	>50						

Table 1. Intersection Level of Service Criteria

Source: 2000 Highway Capacity Manual, Transportation Research Board

In addition to delay and LOS, the operational capacity and vehicular queues are calculated and used to further quantify traffic operations at intersections. The following describes these other calculated measures.

The volume-to-capacity (v/c) ratio is a measure of congestion at an intersection approach. A v/c ratio below one indicates that the intersection approach has adequate capacity to process the arriving traffic volumes over the course of an hour. A v/c ratio of one or greater indicates that the traffic volume on the intersection approach exceeds capacity.

The 95th percentile queue, measured in feet, denotes the farthest extent of the vehicle queue (to the last stopped vehicle) upstream from the stop line. This maximum queue occurs five percent, or less, of the time during the peak hour and typically does not develop during off-peak hours. Since volumes fluctuate throughout the hour, the 95th percentile queue represents what can be considered a "worst case" condition. Queues at an intersection are generally below the 95th percentile length throughout most of the peak hour. It is also unlikely that 95th percentile queues for each approach to an intersection occur simultaneously.

Table 2 presents the Existing (2015) Condition intersection capacity analysis for the study area intersection during the weekday a.m. and p.m. peak hours. The detailed analysis sheets are provided in the **Appendix**.



				41-	A1-
Intersection	LOS	Delay	V/C Ratio	50 ^m Percentile Queue (feet)	95 th Percentile Queue (feet)
a.m	. Peak H	lour			
Centre Street/Willow Street	С	21.8	-	-	-
Centre Street eastbound thru thru	С	24.8	0.38	134	183
Centre Street eastbound right	С	24.0	0.08	21	48
Centre Street westbound left	В	14.8	0.14	21	48
Centre Street westbound thru thru	В	14.8	0.28	96	146
Willow Street northbound left	D	47.4	0.34	26	57
Willow Street northbound right	А	5.9	0.17	0	21
Willow Street southbound left/thru/right	D	51.8	0.48	44	81
p.m	. Peak H	lour			
Centre Street/Willow Street	С	24.9	-	-	-
Centre Street eastbound thru thru	С	31.0	0.45	170	228
Centre Street eastbound right	С	29.5	0.15	40	80
Centre Street westbound left	В	15.1	0.25	42	83
Centre Street westbound thru thru	В	15.9	0.37	138	205
Willow Street northbound left	Е	63.3	0.57	46	89
Willow Street northbound right	А	5.8	0.23	0	38
Willow Street southbound left/thru/right	Е	58.3	0.56	63	110

 $Grey\ shading\ indicated\ LOS\ of\ E\ or\ F$

The intersection of Centre Street/Willow Street currently operates at LOS C during both the weekday a.m. and p.m. peak hours. The longest queues at the intersection occur along Centre Street during the peak hours, with 95th percentile queues ranging from 146 feet to 183 feet (approximately 6 to 8 vehicles) during the a.m. peak hour and 205 feet to 228 feet (approximately 9 to 10 vehicles) during the p.m. peak hour.

Existing Public Transportation

The Project site is located in the vicinity of the MBTA Commuter Rail Needham Line and three bus lines that have stops along Centre Street. The nearby public transportation services are shown in **Figure 2**.

COMMUTER RAIL NEEDHAM LINE

The Needham Line of the MBTA commuter rail system stops at Highland Station, approximately 1,500 feet from the Project site, and Bellevue Station, approximately 2,000 feet from the site. The Needham Line runs between South Station in Boston and Needham Heights. The Needham Line operates with weekday service from 6:05 a.m. to 11:10 p.m. with approximately 30 minute peak hour headways. Saturday service runs from 7:10 a.m. to 12:00 a.m. with 120 minute headways.

MBTA BUS LINES

There are three bus lines that operate in the vicinity of the Project that provide service to/from Forest Hills Station, which serves the MBTA Orange Line. The inbound bus stop is located at the southeast corner of Centre Street/Corey Street and the outbound bus stop is located at the northeast corner of Centre Street/Willow Street. The three bus lines are highlighted in **Table 3**.

Bus Number	Bus Route	Approximate Peak Headway (min)
35	Dedham Mall/Stimson Street – Forest Hills Station	15
36	Charles River Loop or VA Hospital - Forest Hills Station	20
37	Baker and Vermont Streets – Forest Hills Station	20

Table 3. MBTA Bus Routes

TECHNICAL MEMORANDUM 1789 Centre Street

Figure 2. *Public Transportation*



HOWARD STEIN HUDSON

Existing Pedestrian and Bicycle Facilities

Pedestrian and bicycle counts at the study area intersection were conducted concurrently with the vehicle counts on June 30, 2015 during the weekday a.m. and p.m. peak periods (7:00-9:00 a.m. and 4:00-6:00 p.m., respectively)

The highest pedestrian volume occurs at the crossing southern crosswalk along Centre Street crossing Willow Street. There were 20 pedestrians observed using this crosswalk during the a.m. peak hour and 31 pedestrians during the p.m. peak hour. The highest bicycle volume occurs at the Centre Street eastbound approach. There were 5 bikes during the a.m. peak hour and 5 bikes during the p.m. peak hour.

Sidewalks in the Project area are in good condition and supply adequate capacity. Handicappedaccessible ramps and crosswalks are provided at all approaches to the study area intersection. There is no on-street bicycle infrastructure provided in the vicinity of the Project. The Existing (2015) Condition bicycle and pedestrian volumes are shown in **Figure 3**.

Existing Car Sharing Services

Car sharing enables easy access to short term vehicular transportation. Vehicles are rented on an hourly or daily basis and all vehicle costs (gas, maintenance, insurance, and parking) are included in the rental fee. Vehicles are checked out for a specific time period and returned to their designated location.

Nearby car sharing services provide an important transportation option and reduce the need for private vehicle ownership. A map of all car sharing locations within the Project site vicinity is shown in **Figure 4**.





Figure 3. Existing (2015) Condition Bicycle and Pedestrian Volumes

TECHNICAL MEMORANDUM 1789 Centre Street

Figure 4. Car Sharing Services



HOWARD STEIN HUDSON

Engineers + Planners

Future Conditions

No-Build Condition

NO-BUILD CONDITION TRAFFIC VOLUMES

The No-Build Condition reflects a future scenario that incorporates any anticipated traffic volume changes independent of the Project and any planned infrastructure improvements that will affect travel patterns throughout the study area. Infrastructure improvements include roadway, public transportation, pedestrian, and bicycle improvements. Background traffic growth is based on two factors: an annual growth rate and growth associated with specific developments near the project. Based on a review of available information provided by the City of Boston, there are no major developments in the vicinity of the Project site that will have a measureable impact on operations at the intersection of Centre Street/Willow Street.

To develop the 2020 No-Build Condition traffic volumes at the study area intersection, a half-percent per year annual growth rate was applied to the 2015 Existing Condition traffic volumes. The 2020 No-Build a.m. and p.m. peak hour traffic volumes are show in **Figure 5**.

NO-BUILD CONDITION TRAFFIC OPERATIONS

The 2020 No-Build Condition operation analysis uses the same methodology as the 2015 Existing Condition operation analysis. **Table 4** presents the 2020 No-Build Condition operations analysis for the a.m. and p.m. peak hours. The detailed analysis sheets are provided in the **Appendix**.

Based on the analysis presented in **Table 4**, the intersection of Centre Street/Willow Street will continue to operate at an overall LOS C during both the weekday a.m. and p.m. peak hours.



Figure 5. No-Build (2020) Condition Traffic Volumes





Intersection	LOS	Delay	V/C Ratio	50 th Percentile Queue (feet)	95 th Percentile Queue (feet)
a.m	n. Peak H	lour	-		
Centre Street/Willow Street	С	23.6	-	-	-
Centre Street eastbound thru thru	С	27.6	0.54	208	275
Centre Street eastbound right	С	24.1	0.08	21	48
Centre Street westbound left	В	15.4	0.19	22	50
Centre Street westbound thru thru	В	15.0	0.28	99	151
Willow Street northbound left	D	47.5	0.35	26	57
Willow Street northbound right	А	6.1	0.17	0	22
Willow Street southbound left/thru/right	D	51.8	0.48	45	83
p.m	n. Peak H	lour			
Centre Street/Willow Street	С	25.3	-	-	-
Centre Street eastbound thru thru	С	31.3	0.47	175	234
Centre Street eastbound right	С	29.6	0.16	41	81
Centre Street westbound left	В	15.4	0.26	44	87
Centre Street westbound thru thru	В	16.2	0.38	144	213
Willow Street northbound left	E	64.5	0.59	48	91
Willow Street northbound right	А	5.8	0.23	0	38
Willow Street southbound left/thru/right	E	58.1	0.57	65	113

Table 4. No-Build (2020) Condition Intersection Operations Analysis

Build Condition

SITE ACCESS AND CIRCULATION

Vehicular access/egress will be provided by a new driveway located off of Willow Street, approximately 90 feet north of Centre Street. The driveway will accommodate two-way travel and provide access to the 29 ground-level parking spaces that will serve the Project. Since Willow Street is one-way in the southbound direction, vehicles will need to access the site by way of one of the cross streets to the north such as Alhambra Road, Schirmer Road, or Weld Street. Due to the size of the Project and its potential trip generating characteristics, this will represent a very minimal increase in traffic in the surrounding neighborhood to the north during the peak hours.

Primary pedestrian access for the residential portion of the Project will be located near the corner of Centre Street/Willow Street and primary pedestrian access for the commercial portion of the Project will be located along Centre Street. Secondary access for pedestrians will be provided along Willow Street and through the parking area. The sidewalks abutting the Project site will be upgraded and/or reconstructed around the perimeter of the site.

An alternative driveway location was also considered throughout the development of the site plan. A driveway location along Centre Street was reviewed and it was determined that the location along Willow Street would be more desirable from a safety and traffic operations perspective. Locating a driveway along Centre Street would introduce additional vehicle-pedestrian conflicts due to the higher pedestrian volumes along Centre Street. Access and egress along Centre Street is also more difficult than at Willow Street due to the need to cross several lanes of traffic for vehicles making a left-turn maneuver into or out of the Project site. Access and egress along Willow Street would be restricted to right-turns only due to the one-way nature of Willow Street. The Willow Street location would also have significantly fewer turning movement conflicts due to the much lower traffic volumes along Willow Street.

TRIP GENERATION

Trip generation is a complex, multi-step process that produces an estimate of vehicle trips, transit trips, walk trips, and bicycle trips associated with a proposed project and a specific land use program. A project's location and proximity to different modes determines how people will travel to and from that project site.



To estimate the number of trips expected to be generated by the Project, data published by the Institute of Transportation Engineers (ITE) in the Trip Generation Manual¹ were used. ITE provides data to estimate the total number of unadjusted vehicular trips associated with the Project. In an urban setting well served by transit, adjustments are necessary to account for other travel mode shares such as walking, bicycling, and transit.

Trip generation estimates for the Project were derived using the following Land Use Codes (LUC):

LUC 220 – Apartment. The apartment land use can be a rental dwelling unit located within the same building with at least three other dwelling units.

LUC 820 – Shopping Center. The shopping center land use is an integrated group of commercial establishments that is planned, developed, owned, and managed as a unit. The trip generation characteristics of this LUC are expected to provide a conservative estimate for the proposed retail space.

MODE SPLIT

The BTD publishes vehicle, transit, and walking mode split rates for different areas of Boston. The Project is located within designated Area 19 – West Roxbury. The BTD's travel mode share data for Area 19 is shown in **Table 5**. The unadjusted vehicular trips were converted to person trips by using vehicle occupancy rates published by the Federal Highway Administration (FHWA)². Since the mode splits shown in **Table 5** are for the entirety of West Roxbury, it is expected that the Project's transit mode share will be higher due to the Project's proximity to good public transportation opportunities for commuting into Boston when compared to other areas of West Roxbury.

¹Trip Generation Manual, 9th Edition; Institute of Transportation Engineers; Washington, D.C.; 2012. ²Summary of Travel Trends: 2009 National Household Travel Survey; FHWA; Washington, D.C.; June 2011.

Land Us	se	Walk/Bike Share ¹	Transit Share ¹	Auto Share ¹	Local Vehicle Occupancy Rate ²
		-	Daily		
Desidential	In	11%	7%	82%	1.13
Residential	Out	11%	7%	82%	1.13
Detail	In	11%	7%	82%	1.78
Retail	Out	11%	7%	82%	1.78
		• •	a.m. Peak Hour		
Desidential	In	13%	7%	80%	1.13
Residential	Out	9%	18%	73%	1.13
Detail	In	13%	7%	80%	1.78
Retail	Out	9%	18%	73%	1.78
		•	p.m. Peak Hour		
Desidential	In	9%	18%	73%	1.13
Residential	Out	13%	7%	80%	1.13
Detail	In	9%	18%	73%	1.78
Retail	Out	13%	7%	80%	1.78

Table 5. | Mode Split

1 Boston Transportation Department mode share data for Area 19.

2 2009 National Household Travel Survey.

TRIP GENERATION SUMMARY

The trip generation process described above yields the trips adjusted by mode split associated with the Project. The Project-generated trips are summarized in **Table 6**, with detailed trip generation information provided in the **Appendix**.



Time Period	Direction	New Vehicular Trips	New Transit Trips	New Walk/Bicycle Trips
	In	60	6	11
Daily	<u>Out</u>	<u>60</u>	<u>6</u>	<u>11</u>
	Total	120	12	22
	In	2	0	0
a.m. Peak Hour	<u>Out</u>	<u>5</u>	<u>1</u>	<u>1</u>
	Total	7	1	1
	In	6	1	1
p.m. Peak Hour	<u>Out</u>	<u>4</u>	<u>0</u>	<u>1</u>
	Total	10	1	2

 Table 6.
 |
 Project Trip Generation

As shown in **Table 6**, the Project is expected to generate approximately 120 new daily vehicle trips, with 7 new vehicle trips (2 entering and 5 exiting) during the weekday a.m. peak hour and 10 new vehicle trips (6 entering and 4 exiting) during the weekday p.m. peak hour. This corresponds to an increase of approximately one new vehicle trip every 8 minutes during the weekday a.m. peak hour and one new vehicle trip every 6 minutes during the weekday p.m. peak hour. These increases fall within the range of typical fluctuations in traffic volumes over the course of the day and during the peak hours.

The Project is also expected to generate approximately 12 new transit trips and 22 new walk/bicycle trips on a daily basis. As previously mentioned, the mode split data used is for the entirety of West Roxbury and it is expected that the Project will have a lower vehicular mode split and higher transit/walk mode splits than what is shown in **Table 5**.

TRIP DISTRIBUTION

The trip distribution identifies the various travel paths for vehicles arriving and leaving the Project Site. Trip distribution patterns for the Project were based on existing roadway volumes at the study area intersection. The trip distribution pattern for the Project is illustrated in **Figure 6**.

The Project-generated vehicle trips were assigned to the study area roadway network based on the trip distribution patterns shown in **Figure 6** and are shown in **Figure 7** for the a.m. and p.m. peak

hours. The Project-generated trips were then added to the 2020 No-Build Condition traffic volumes to develop the 2020 Build Condition peak hour traffic volume networks and are shown in **Figure 8** for the a.m. and p.m. peak hours.

BUILD CONDITION TRAFFIC OPERATIONS

The 2020 Build Condition operations analysis use the same methodology as the 2015 Existing and 2020 No-Build Condition operations analysis. The results of the 2020 Build conditions traffic analysis at study area intersection is presented in **Table 7** for the a.m. and p.m. peak hours. The detailed analysis sheets are provided in the **Appendix**.

Intersection	LOS	Delay	V/C Ratio	50 th Percentile Queue (feet)	95 th Percentile Queue (feet)
a.m	. Peak H	lour			
Centre Street/Willow Street	С	23.9	-	-	-
Centre Street eastbound thru thru	С	27.6	0.54	208	275
Centre Street eastbound right	С	24.1	0.08	21	48
Centre Street westbound left	В	15.8	0.19	22	51
Centre Street westbound thru thru	В	15.3	0.29	101	154
Willow Street northbound left	D	47.3	0.35	26	57
Willow Street northbound right	А	6.1	0.17	0	22
Willow Street southbound left/thru/right	D	52.2	0.51	49	88
p.m	. Peak H	lour			
Centre Street/Willow Street	С	25.5	-	-	-
Centre Street eastbound thru thru	С	31.3	0.47	175	234
Centre Street eastbound right	С	29.7	0.16	41	81
Centre Street westbound left	В	15.6	0.27	44	87
Centre Street westbound thru thru	В	16.4	0.38	146	215
Willow Street northbound left	Е	64.6	0.59	48	92
Willow Street northbound right	А	5.8	0.23	0	38
Willow Street southbound left/thru/right	Е	58.8	0.59	68	118

Table 7. Build (2020) Condition Intersection Operations Analysis

TECHNICAL MEMORANDUM 1789 Centre Street

Figure 6. Vehicle Trip Distribution





Figure 7. *Project Generated Vehicle Trips*





Figure 8. Build (2020) Condition Traffic Volumes



As shown in **Table 7**, the intersection of Centre Street/Willow Street will continue to operate at LOS C during both the weekday a.m. and p.m. peak hours with the occupancy of the Project. The Project is expected to have little impact on operations at the signal intersection due to its minimal trip generating characteristics. The Project will also have minimal impact upon the surrounding roadway network including Alhambra Road, Schirmer Road, and Weld Street. The surrounding roadway network will see a slight, but mostly unperceivable, increase in traffic volumes from the Project and has the operating capacity to accommodate the Project-generated trips without the need for any additional improvements.

PARKING

The Project will provide a total of 29 parking spaces on the site. A total of 27 parking spaces will be provided for the residences on the site and two parking spaces will be provided for the commercial space. The on-site parking supply is consistent with the requirements of the zoning regulations for this site.

Conclusion

Based on the evaluation presented in this memorandum, the Project at 1789 Centre Street will have minimal impact on the surrounding transportation infrastructure. The Project is expected to generate ten or fewer trips per hour during the peak commuter periods. This is the equivalent of one additional vehicle every six minutes, which is within the range of typical fluctuations in daily and hourly traffic volumes. The surrounding transportation infrastructure has the capacity to accommodate the minimal amount of additional trips expected to be generated by the Project without the need for any additional improvements.



Appendix

TRAFFIC VOLUME COUNTS

TRIP GENERATION CALCULATIONS

INTERSECTION CAPACITY ANALYSIS WORKSHEETS

11 BEACON STREET, SUITE 1010 | BOSTON, MASSACHUSETTS 02108 | 617.482.7080



TRAFFIC VOLUME COUNTS

11 BEACON STREET, SUITE 1010 | BOSTON, MASSACHUSETTS 02108 | 617.482.7080

				1173	1240	1243	1252	1221								
				1132	1188	1188	1185	1149								
			Left	0	0	0	0	0	0	0	0	0		%0		
	Centre Street	From West	Thru	94	105	119	125	118	102	108	100	453	0.90	3%		
)		Right	8	8	13	12	9	10	14	18	42		7%		
			Left	7	17	10	5	ω	11	14	ω	38		5%		
	Villow Stree	Willow Stree From South	Thru	0	0	0	0	0	0	0	0	0	0.88	%0		
Wi	Λ		Right	14	13	17	16	14	12	12	15	54		2%		
	t	entre Street From East	Left	11	15	13	1	19	14	20	17	64		%0		
	entre Stree		Thru	94	100	119	130	116	106	120	112	472	0.95	4%		
an	ŭ		Right	0	0	0	0	0	0	0	0	0		%0		
	Willow Street	fillow Street From North		t	Left	9	9	9	7	5	2	9	5	20		%0
			Thru	4	-	4	2	2	4	ო	-	11	0.86	%0		
TITITIC 1.		-	Right	e	7	9	ი	ი	9	7	5	31		3%		
5			Start Time	07:00 AM	07:15 AM	07:30 AM	07:45 AM	08:00 AM	08:15 AM	08:30 AM	08:45 AM	Volume	PHF	HV%		

File Name: C:\TMC 2015\04584A.ppd

Start Date: 6/30/2015 Start Time: 7:00:00 AM Site Code: 2015089 Comment 1: N/S: Willow Street Comment 2: E/W: Centre Street

		Willow : From N	Street Vorth			Centre From I	Street East			Willow S From S	Street			Centre Si From W	treet est	
	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds
07:00 AM	0	0	0	e	0	-	0	0	0	0	0	e	0	-	0	-
07:15 AM	0	0	0	4	0	-	0	0	0	0	-	4	0	-	0	4
07:30 AM	0	0	0	2	0	0	0	0	0	0	0	2	0	ო	0	-
07:45 AM	0	0	0	-	0	-	0	0	0	0	0	4	0	0	0	n
08:00 AM	0	0	0	ω	0	0	0	0	0	0	0	e	0	-	0	8
08:15 AM	0	0	0	4	0	0	0	0	0	0	0	7	-	-	0	5
08:30 AM	0	0	0	2	0	0	0	2	0	0	0	9	0	2	0	3
08:45 AM	0	0	0	7	0	-	0	-	0	0	0	7	0	-	0	0
Volume	0	0	0	20	0	-	0	2	0	0	0	20	~	4	0	19

41 55 67 72

			1518	1513	1567	1618	1605					
			1451	1437	1486	1521	1510					
		Left	0	0	0	0	0	0	0	0	0	%0
	entre Stree ⁻ rom West	Thru	129	96	116	136	129	128	112	116	505 0.94	3%
	0	Right	23	20	19	17	17	19	18	23	71	4%
		Left	15	19	14	15	16	16	15	20	62	2%
	'illow Street rom South	Thru	0	0	0	0	0	0	0	0	0 0.92	%0
	ŚШ	Right	26	17	19	22	29	23	29	26	103	%0
		Left	22	17	23	27	30	30	24	23	111	%0
	entre Stree From East	Thru	154	131	144	146	136	132	170	143	584 0.90	3%
ry, MA	S	Right	0	0	0	0	0	0	0	0	0	%0
r w Street re Street W. Roxbu		Left	4	11	9	9	2	10	6	10	27	%0
6/30/2015 4:00:00 PM 2015089 N/S: Willo E/W: Cent City, State: Client: HSI	/illow Street ⁻ rom North	Thru	7	4	5	9	ი	4	9	2	25 0.92	%0
Start Date: tart Time: Site Code omment 1: omment 2: omment 3: omment 4:	5 -	Right	10	5	б	11	ω	7	7	12	33	%0
∞ ° ° ° ° ° ° ° °		Start Time	04:00 PM	04:15 PM	04:30 PM	04:45 PM	05:00 PM	05:15 PM	05:30 PM	05:45 PM	Volume PHF	%ЛН

File Name: C:\TMC 2015\04584AA.ppd

		Willow From I	Street North			Centre S From F	Street East			Willow S From S	Street			Centre From V	Street Vest	
	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds
04:00 PM	0	0	0	-	0	0	0	2	0	0	0	4	0	0	0	4
04:15 PM	0	0	0	2	0	-	0	-	0	0	0	8	0	-	0	2
04:30 PM	0	0	0	6	0	с	0	0	0	0	0	5	0	0	0	-
04:45 PM	0	0	0	9	0	2	0	0	0	0	0	5	0	-	0	6
05:00 PM	0	0	-	റ	0	-	0	-	0	0	0	4	-	2	0	-
05:15 PM	0	0	0	5	0	0	0	-	0	0	0	7	-	0	0	4
05:30 PM	0	0	-	7	0	-	0	9	0	0	0	15	0	0	0	4
05:45 PM	-	0	0	9	0	~	0	~	0	0	0	10	0	0	0	0
Volume	0	0	2	27	0	9	0	8	0	0	0	31	2	r	0	18

67 76 81 97 95



TRIP GENERATION CALCULATIONS

11 BEACON STREET, SUITE 1010 | BOSTON, MASSACHUSETTS 02108 | 617.482.7080

1789 Centre Street - West Roxbury, MA Trip Generation Assessment Project Number 2015089.00 HOWARD/STEIN-HUDSON ASSOCIATES 23-Jul-15

			Trip Rates (Trips/ksf or	Unadjusted	Internal		Less capture	Assumed national vehicle	Converted to	Transit	Transit	Walk/Bike/	Walk/ Bike/	Vehicle	Total Vehicle	Assumed local auto occupancy	Total Adjusted
Land Use	Size	Category	unit)	Vehicle Trips	trips	Pass-by %	trips	occupancy rate Dailv Peak Ho	Person trips	Share ⁵	Trips	Other Share [±]	Other Trips	Share≏	Person Trips	rate for autos [°]	Auto Trips
Residential ⁴	19	Total	5.79	110	%0	%0	110	1.13	124		8		14		102	1.13	6
	units	Ч	2.89	55	%0	%0	55	1.13	62	7%	4	11%	7	82%	51	1.13	45
		Out	2.89	55	%0	%0	55	1.13	62	7%	4	11%	7	82%	51	1.13	45
Retail ⁵	0.84	Total	42.86	36	%0	%0	36	1.78	64		4		8		52	1.78	30
	KSF	Ē	21.43	18	%0	%0	18	1.78	32	7%	2	11%	4	82%	26	1.78	15
		Out	21.43	18	%0	%0	18	1.78	32	7%	2	11%	4	82%	26	1.78	15
				146					188		12		22				120
				73					94		9		4				09
				73					94		9		4				09
								AM Peak Hou	5								
Residential ⁴	19	Total	0.42	8	%0	%0	8	1.13	6		1		+		7	1.13	9
	units	드	0.05	-	%0	%0	-	1.13	-	7%	0	13%	0	80%	-	1.13	-
		Out	0.37	7	%0	%0	7	1.13	8	18%	1	6%	1	73%	9	1.13	5
Retail ⁵	0.84	Total	1.19	۲	%0	%0	+	1.78	2		0		0		2	1.78	١
	KSF	드	1.19	-	%0	%0	-	1.78	2	2%	0	13%	0	80%	7	1.78	-
		Out	0.00	0	%0	%0	0	1.78	0	18%	0	9%	0	73%	0	1.78	0
				6					11		1		٢				7
				2					3		0		0				2
				7					8		1		1				5
								PM Peak Hou									
Residential ⁴	19	Total	0.53	10	%0	%0	10	1.13	11		٢		٢		8	1.13	7
	units	드	0.37	7	%0	%0	7	1.13	80	18%	-	%6	۰	73%	9	1.13	5
		Out	0.16	3	%0	%0	3	1.13	3	7%	0	13%	0	80%	2	1.13	2
Retail ⁵	0.84	Total	3.57	З	%0	%0	3	1.78	9		0		1		5	1.78	3
	KSF	드	1.19	-	%0	%0	-	1.78	2	18%	0	%6	0	73%	-	1.78	-
		Out	2.38	2	%0	%0	2	1.78	4	7%	0	13%	1	80%	3	1.78	2
				13					17		-		2				10
				8					10		٦		-				9
				L					٢		-		Ţ				•

2009 National vehicle occupancy rates - 1.13:home to work; 1.84: family/personal business; 1.78: shopping; 2.2 social/recreational
 Mode shares based on peak-hour BTD Data for Area 19
 Local vehicle occupancy rates based on 2009 National vehicle occupancy rates.
 Local vehicle occupancy rate, 9th Edition, LUC 230 (Residential Condominium/Townhouse), average rate



INTERSECTION CAPACITY ANALYSIS WORKSHEETS

11 BEACON STREET, SUITE 1010 | BOSTON, MASSACHUSETTS 02108 | 617.482.7080

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Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR	ø2		
Lane Configurations		44	1	5	44		5		1		4		~-		
Volume (vph)	0	453	42	64	472	0	38	0	54	20	11	31			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Storage Length (ft)	0		120	125		0	0		80	0		0			
Storage Lanes Tapor Longth (ft)	25		1	20		0	100		1	25		0			
Lane Util Factor	1 00	0.95	1.00	1 00	0.95	1.00	1.00	1.00	1.00	1 00	1.00	1 00			
Ped Bike Factor	1.00	0.70	0.98	1.00	0.70	1.00	1100	1100	1.00	1.00	1.00	1100			
Frt			0.850						0.850		0.932				
Flt Protected				0.950			0.950				0.984				
Satd. Flow (prot)	0	3110	1358	1624	3080	0	1392	0	1275	0	1383	0			
Fit Permitted	0	2110	1225	0.420	2000	0	0.798	0	1075	0	0.984	0			
Right Turn on Red	0	3110	No	/10	3000	No	1170	0	Yes	0	1303	No			
Satd. Flow (RTOR)						110			76						
Link Speed (mph)		30			30			30			30				
Link Distance (ft)		513			640			311			412				
Travel Time (s)		11.7			14.5			7.1			9.4				
Conti. Bikes (#/nr) Peak Hour Factor	0.00	0 00	4	0.05	0.05	0.95	0.88	0.88	0.88	0.86	0.86	0.86			
Heavy Vehicles (%)	0.70	3%	7%	0%	4%	0.75	5%	0%	2%	0.00	0%	3%			
Bus Blockages (#/hr)	0	7	0	0	7	0	0	0	0	0	0	0			
Parking (#/hr)							0	0	1	1	1	1			
Adj. Flow (vph)	0	503	47	67	497	0	43	0	61	23	13	36			
Shared Lane Traffic (%)	0	502	47	(7	407	0	10	0	(1	0	70	0			
Lane Group Flow (vpn)	0	503	4/ Porm	6/	497	0	43 D Dm	0	61	Dorm	/2	0			
Protected Phases		1	Feili	D.r +r 4	14		D.FIII		4	Feili	3		2		
Permitted Phases			1	1			3		3	3	0		-		
Detector Phase		1	1	4	14		3		4	3	3				
Switch Phase															
Minimum Initial (s)		10.0	10.0	6.0			8.0		6.0	8.0	8.0		4.0		
Minimum Split (s)		16.0	16.0	11.0			14.0		11.0	14.0	14.0		28.0		
Total Split (S) Total Split (%)		39.0	39.0	11.0%			22.0		11.0%	22.0	22.0		28.0		
Maximum Green (s)		33.0	33.0	6.0			16.0		6.0	16.0	16.0		24.0		
Yellow Time (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0		
All-Red Time (s)		3.0	3.0	2.0			3.0		2.0	3.0	3.0		1.0		
Lost Time Adjust (s)		0.0	0.0	0.0			0.0		0.0		0.0				
Total Lost Time (s)		6.0	6.0	5.0			6.0		5.0	Load	6.0		Log		
Leau/Lag		Yes	Yes	Yes			Yes		Yes	Yes	Yes		Yes		
Vehicle Extension (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0		
Recall Mode		C-Max	C-Max	None			None		None	None	None		None		
Walk Time (s)													7.0		
Flash Dont Walk (s)													17.0		
Act Effet Groop (s)		12.0	12.0	525	50 7		10.0		22.6		10.0		01		
Actuated g/C Ratio		43.0	43.0	0.54	0.59		0.11		0.24		0.11				
v/c Ratio		0.38	0.08	0.14	0.28		0.34		0.17		0.48				
Control Delay		24.8	24.0	14.8	14.8		47.4		5.9		51.8				
Queue Delay		0.0	0.0	0.0	0.0		0.0		0.0		0.0				
Total Delay		24.8	24.0	14.8	14.8		47.4		5.9		51.8				
LOS Approach Dolay		24.0	С	В	14.0		D		A		D 51.0				
Approach LOS		24.0 C			14.0 B						51.6 D				
Queue Length 50th (ft)		134	21	21	96		26		0		44				
Queue Length 95th (ft)		183	48	48	146		57		21		81				
Internal Link Dist (ft)		433			560			231			332				
Turn Bay Length (ft)		100/	120	125	1007		107		80		001				
Base Capacity (Vpn) Stanuation Can Roductn		1330	569	470	1807		187		359		221				
Spillback Cap Reductn		0	0	0	0		0		0		0				
Storage Cap Reductn		0	0	0	0		0		0		0				
Reduced v/c Ratio		0.38	0.08	0.14	0.28		0.23		0.17		0.33				
Intersection Summary															
Area Type: CBE)														
Cycle Length: 100	-														
Actualeu Cycle Length: 100 Offset: 80 (80%), Referenced to pl	hase 1.E	RWR Sta	rt of Green	n											
Natural Cycle: 70	nase lit		IT OF GIER												
Control Type: Actuated-Coordinate	ed														
Maximum v/c Ratio: 0.48															
Intersection Signal Delay: 21.8				Inte	ersection I	LOS: C									
Intersection Capacity Utilization 43	3.7%			ICI	J Level of	Service A									
Analysis Period (min) 15															
Splits and Phases: 3: Willow Str	eet & C	entre Stre	et				20						- A.	 4	-
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ane Groun	FBI	FRT	FRR	WBI	WRT	WBR	NRI	NBT	NBR	SRI	SBT	SBR	a2
Lane Configurations	LDL	*		WDL	**	WDR	NDL	IND I	NDK 7	JDL	301 4	JDK	WZ.
Volume (vph)	0	505	71	111	584	0	62	0	103	27	25	33	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0		120	125		0	0		80	0		0	
Storage Lanes	25		1	20		0	100		1	25		0	
Lane Litil Factor	1 00	0.95	1.00	1 00	0.95	1.00	1 00	1.00	1 00	1 00	1.00	1.00	
Ped Bike Factor	1.00	0.70	0.98	1100	0.70	1.00	1100	1100	1100	1100	1.00	1.00	
Frt			0.850						0.850		0.947		
Flt Protected				0.950			0.950				0.984		
Satd. Flow (prot)	0	3154	1358	1624	3110	0	1433	0	1301	0	1426	0	
Fit Permitted	0	2154	100/	0.3/5	2110	0	0.683	0	1201	0	0.984	0	
Salu. Flow (perifi) Dight Turn on Pod	0	3154	1320 No	041	3110	No	1031	0	1301 Voc	U	1420	No	
Satd Flow (RTOR)			NU			NU			112			NU	
Link Speed (mph)		30			30			30	112		30		
Link Distance (ft)		513			640			311			412		
Travel Time (s)		11.7			14.5			7.1			9.4		
Confl. Bikes (#/hr)			3										
Peak Hour Factor	0.94	0.94	0.94	0.90	0.90	0.90	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Venicles (%)	0%	3%	4%	0%	3%	0%	2%	0%	0%	0%	0%	0%	
Dus Blockages (#/NF) Parking (#/hr)	0	U	/	U	/	U	0	0	1	1	1	1	
Adi Flow (vpb)	0	537	76	123	649	0	67	0	112	29	27	36	
Shared Lane Traffic (%)	0		10	125	047	0	07	U	112	27	21	30	
Lane Group Flow (vph)	0	537	76	123	649	0	67	0	112	0	92	0	
Turn Type		NA	Perm	D.P+P	NA		D.Pm		pm+ov	Perm	NA		
Protected Phases		1		4	14				4		3		2
Permitted Phases			1	1			3		3	3			
Detector Phase		1	1	4	14		3		4	3	3		
SWITCH PHASE		10.0	10.0	4.0			0.0		4.0	0.0	0.0		4.0
Minimum Split (s)		10.0	16.0	0.0			8.0 14.0		0.0	8.0 14.0	8.0 14.0		4.0 28.0
Total Split (s)		38.0	38.0	18.0			26.0		18.0	26.0	26.0		28.0
Total Split (%)		34.5%	34.5%	16.4%			23.6%		16.4%	23.6%	23.6%		25%
Maximum Green (s)		32.0	32.0	13.0			20.0		13.0	20.0	20.0		24.0
Yellow Time (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0
All-Red Time (s)		3.0	3.0	2.0			3.0		2.0	3.0	3.0		1.0
Lost Time Adjust (s)		0.0	0.0	0.0			0.0		0.0		0.0		
Total Lost Time (s)		6.0	6.0	5.0			6.0		5.0	Lood	6.0		1.00
Ledu/Ldy		Lead Voc	Lead Voc	Lag			Lead Voc		Lag	Lead Voc	Lead		Ldg Voc
Vehicle Extension (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0
Recall Mode		C-Max	C-Max	None			None		None	None	None		None
Walk Time (s)													7.0
Flash Dont Walk (s)													17.0
Pedestrian Calls (#/hr)													84
Act Effct Green (s)		41.4	41.4	58.9	62.9		12.7		35.2		12.7		
Actuated g/C Ratio		0.38	0.38	0.54	0.57		0.12		0.32		0.12		
Control Delay		31.0	20.15	0.25	15.0		63.3		5.8		58.3		
Oueue Delay		0.0	0.0	0.0	0.0		0.0		0.0		0.0		
Total Delay		31.0	29.5	15.1	15.9		63.3		5.8		58.3		
LOS		С	С	В	В		E		А		E		
Approach Delay		30.8			15.8						58.3		
Approach LOS		С			В				_		E		
Queue Length 50th (ft)		170	40	42	138		46		0		63		
Queue Length 95th (tt)		228	80	83	205		89	111	38		110		
Turn Bay Longth (ft)		433	120	105	000			231	00		332		
Base Capacity (vnh)		1187	499	499	1761		187		498		259		
Starvation Cap Reductn		0	477	477	0		0		470		2.37		
Spillback Cap Reductn		0	0	0	0		0		0		0		
Storage Cap Reductn		0	0	0	0		0		0		0		
Reduced v/c Ratio		0.45	0.15	0.25	0.37		0.36		0.22		0.36		
Intersection Summary													
Area Type	CBD												
Cycle Length: 110	000												
Actuated Cycle Length: 110													
Offset: 0 (0%), Referenced t	o phase 1:EB	WB, Start	of Green										
Natural Cycle: 70													
Control Type: Actuated-Coo	rdinated												
Maximum v/c Ratio: 0.57	10				0000-11-	100.0							
Intersection Signal Delay: 24 Intersection Canacity Littlend	4.9 tion 49 E ^{0/}			In	ersection	LUS: C							
Analysis Period (min) 15	1011 40.3%			IC.	o Level O	Service A	۱						
maysis rendu (IIIII) 13													
Splits and Phases: 3: Will	ow Street & C	Centre Stre	et										
	Succi a G	5	- •			2.4							
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ane Group	FBI	FRT	FRR	WRI	WRT	WBR	NRI	NBT	NRR	SBI	SBT	SBR	ø2
ane Configurations	LDL	A	LDK 7	WDL	**	WDR	NDL	IND I	NDK 7	JDL	301 4	JUK	WZ.
/olume (vph)	0	646	43	66	484	0	39	0	55	21	11	32	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0		120	125		0	0		80	0		0	
Siorage Lanes Faper Length (ft)	25		1	30		U	100		1	25		U	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor			0.98								-		
Frt			0.850						0.850		0.932		
Fit Protected	-	0110	1050	0.950	2000		0.950	^	1075	-	0.984	-	
Satd. Flow (prot)	0	3110	1358	1624	3080	0	1392	0	1275	0	1383	0	
Satd Flow (perm)	0	3110	1325	0.289	3080	0	0.787	0	1275	0	0.984	0	
Right Turn on Red	0	5110	No	171	3000	No	1104	0	Yes	0	1505	No	
Satd. Flow (RTOR)									76				
Link Speed (mph)		30			30			30			30		
Link Distance (ft)		513			640			311			412		
Travel Time (s)		11.7			14.5			7.1			9.4		
Confl. Bikes (#/hr)	0.00	0.00	4	0.05	0.05	0.05	0.00	0.00	0.00	0.07	0.07	0.0/	
Heavy Vehicles (%)	0.90	3%	0.90	0.95	0.95	0.95	5%	0.88 0%	0.88	08.0	0.80	3%	
Bus Blockages (#/hr)	0	7	0	0	7	0	0	0	0	0	0	0	
Parking (#/hr)				Ŭ		v	0	0	1	1	1	1	
Adj. Flow (vph)	0	718	48	69	509	0	44	0	62	24	13	37	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	718	48	69	509	0	44	0	62	0	74	0	
Protected Phases		NA 1	Perm	U.P+P	NA 1.4		D.PM		pm+ov	Perm	NA 2		2
Permitted Phases		1	1	4	14		2		4	2	3		2
Detector Phase		1	1	4	14		3		4	3	3		
Switch Phase							Ŭ			Ū	Ŭ		
Minimum Initial (s)		10.0	10.0	6.0			8.0		6.0	8.0	8.0		4.0
Minimum Split (s)		16.0	16.0	11.0			14.0		11.0	14.0	14.0		28.0
Total Split (s)		39.0	39.0	11.0			22.0		11.0	22.0	22.0		28.0
Total Split (%) Maximum Groon (s)		39.0%	39.0%	11.0%			22.0%		11.0%	22.0%	22.0%		28%
Vellow Time (s)		30	30	3.0			3.0		3.0	3.0	3.0		24.0
All-Red Time (s)		3.0	3.0	2.0			3.0		2.0	3.0	3.0		1.0
Lost Time Adjust (s)		0.0	0.0	0.0			0.0		0.0		0.0		
Total Lost Time (s)		6.0	6.0	5.0			6.0		5.0		6.0		
Lead/Lag		Lead	Lead	Lag			Lead		Lag	Lead	Lead		Lag
Lead-Lag Optimize?		Yes	Yes	Yes			Yes		Yes	Yes	Yes		Yes
Venicle Extension (s)		3.0	3.0	3.0 Nono			3.0 Nono		3.0 Nono	3.0 Nono	3.0		3.0
Walk Time (s)		C-IVIAX	C-IVIAX	None			None		None	None	None		7.0
Flash Dont Walk (s)													17.0
Pedestrian Calls (#/hr)													61
Act Effct Green (s)		43.0	43.0	53.3	58.5		11.1		23.6		11.1		
Actuated g/C Ratio		0.43	0.43	0.53	0.58		0.11		0.24		0.11		
v/c Ratio		0.54	0.08	0.19	0.28		0.35		0.17		0.48		
Control Delay		27.6	24.1	15.4	15.0		47.5		0.1		51.8		
Total Delay		27.6	24.1	15.4	15.0		47.5		6.1		51.8		
LOS		27.0 C	24.1 C	B	13.0 B		47.5 D		A		D		
Approach Delay		27.4	5	_	15.1		_				51.8		
Approach LOS		С			В						D		
Queue Length 50th (ft)		208	21	22	99		26		0		45		
Queue Length 95th (ft)		275	48	50	151		57	0.04	22		83		
Internal Link Dist (ft)		433	120	105	560			231	00		332		
Rase Canacity (vnh)		1336	560	360	1802		18/		350		221		
Starvation Can Reductn		1330	0	309	0		0		337		221		
Spillback Cap Reductn		0	0	0	0		0		0		0		
Storage Cap Reductn		0	0	0	0		0		0		0		
Reduced v/c Ratio		0.54	0.08	0.19	0.28		0.24		0.17		0.33		
Intersection Summary													
Area Type:	CBD												
Cycle Length: 100													
Actuated Cycle Length: 100													
Offset: 80 (80%), Referenced	d to phase 1:I	EBWB, Sta	art of Gree	n									
Natural Cycle: 75 Control Type: Actuated Coor	rdinatod												
Maximum v/c Ratio: 0.54	uillateu												
Intersection Signal Delay: 23	8.6			In	tersection	LOS: C							
Intersection Capacity Utilizat	tion 49.8%			IC	U Level o	Service A	1						
Analysis Period (min) 15													
Callie and Disc. 0. 1177													
Splits and Phases: 3: Willo	ow Street & C	entre Stre	et										
= a1 (R)							1.0						· ·

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Back Group EDI EDI EDI EDI EDI MAI MAI <th< th=""><th></th><th>٦</th><th></th><th>\mathbf{x}</th><th><</th><th>+</th><th>•</th><th>•</th><th>Ť</th><th>~</th><th>1</th><th>Ļ</th><th>1</th><th></th></th<>		٦		\mathbf{x}	<	+	•	•	Ť	~	1	Ļ	1	
Description Description Product of the second sec	ane Group	FBI	FRT	FBR	WBI	WRT	WBR	NBI	NRT	NBR	SRI	SRT	SBR	a2
name (page) name (page) name (page) strange (new) 0 100 100 100 100 100 100 100 100 100 1	Lane Configurations	LDL	44	1	NDL N	**	WDI	NDL NDL	NDT	1001	JDL	4	JUK	102
bala Flav (proph) bala Flav (Volume (vph)	0	518	73	114	599	0	64	0	106	28	26	34	
Stange Long http: 0 0 120 125 0 0 0 80 0 0 0 1 0 1 0 10 100 100 100 1	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Starge Langh 10, 21 30, 10, 10, 100, 100, 100, 100, 100, 1	Storage Length (ft)	0		120	125		0	0		80	0		0	
aper Langh (1) 2, 2 0, 3 0, 0 0, 0 0, 0 0, 0 0, 0 0, 0	Storage Lanes	0		1	1		0	1		1	0		0	
abs bit in Statu index index index index index index index if in 0.89 0.950 0.950 0.950 0.950 0.947 if in 0.351 1.35 1.54 1.35 0.143 0.130 0.142 0.944 if in 0.351 1.35 0.44 1.00 0.944 0.944 if in 0.355 0.365 0.674 0.131 0.142 0.944 if in 0.365 0.64 310 0.142 0.944 0.94 if in 0.117 1.45 7.71 9.4 0.944 0.95 0.95 0.95 0.95 0.95 0.95	Laper Length (II)	25	0.05	1.00	30	0.05	1.00	100	1.00	1.00	25	1.00	1.00	
non- state 0.800 (1) 0.950 (2) 0.92 (2) 0.92 (2) <th0.92 (2) 0.93 (2) 0.93</th0.92 	Pod Riko Factor	1.00	0.95	0.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
if protected 0.990 0.990 0.990 0.994 0.994 if protected 0.314 328 310 0.433 0.1301 0.944 0.944 if the mitted 0.315 132 0.0474 0.1301 0.944 0.944 if the mitted 0.315 130 0.9474 0.1301 0.942 0.944 if the mitted 0.94 0.44 0.90 0.90 0.90 0.92 0.93 0.93 0.93	Frt			0.850						0.850		0.947		
State Flow (prof) 0 3154 1326 1624 310 0 1423 0 1301 0 1426 0 state Flow (perm) 0 3154 1326 624 310 0 1017 0 1301 0 1426 0 0674 0 0674 0 0 0 115 0 115 0 0 0 0 0 0 0 0 0 115 0	Flt Protected				0.950			0.950				0.984		
The Permitted 0.365 0.674 0.984 Staff Evor (PTO) 0 1017 0 1017 0 1017 0 Staff Evor (PTO) 0 1017 0 1017 0 1017 0 Staff Evor (PTO) 0 0 0 0 0 0 0 0 Staff Evor (PTO) 0 0 0 0 0 0 0 0 Staff Evor (PTO) 0 0 0 0 0 0 0 0 0 Staff Evor (PTO) 0 0 0 0 0 0 0 0 0 Staff Evor (PTO) 0 7 0 7 0 0 0 0 0 0 Staff Evor (PTO) 0 7 7 66 0 0 1 1 1 Staff Evor (PTO) 0 551 78 127 666 0 0 115 0 95 0 Staff Evor (PTO) 0 551 78 127 666 0 0 116 14 3 3 2 Staff Evor (PTO) 0 551 78 127	Satd. Flow (prot)	0	3154	1358	1624	3110	0	1433	0	1301	0	1426	0	
state Power No 120 0 120 0 120 0 120 0 state No No Yes No Yes No No </td <td>Flt Permitted</td> <td></td> <td></td> <td></td> <td>0.365</td> <td></td> <td></td> <td>0.674</td> <td></td> <td></td> <td></td> <td>0.984</td> <td></td> <td></td>	Flt Permitted				0.365			0.674				0.984		
Sigh Linn on Red No No Yes No No Yes No No Yes No No No Yes No No No Yes No	Satd. Flow (perm)	0	3154	1326	624	3110	0	1017	0	1301	0	1426	0	
Sale Lino (K1 Ok) 0 0 0 0 0 0 ark Distance (R) 513 640 311 412 ark Distance (R) 513 640 311 412 ark Distance (R) 0 0 94 94 ark Distance (R) 0 94 94 94 ark Distance (R) 0 94 94 94 ark Distance (R) 0 94 94 94 ark Distance (R) 0 0 97 0 0 0 ark Distance (R) 0 0 7 0 0 0 0 biss Biochages (Mn) 0 0 551 78 127 666 0 70 0 115 0 95 are Group Flow (vph) 0 551 78 127 666 0 70 0 115 0 95 are Group Flow (vph) 0 551 78 127 666 0 70 0 115 0 95 are Group Flow (vph) 0 551 78 14 4 3 3 2 are Group Flow (vph) 0 50 660 80 <td>Right Turn on Red</td> <td></td> <td></td> <td>No</td> <td></td> <td></td> <td>No</td> <td></td> <td></td> <td>Yes</td> <td></td> <td></td> <td>No</td> <td></td>	Right Turn on Red			No			No			Yes			No	
mk speak (ma) 30 30 30 30 30 30 30 30 30 30 30 30 30	Satd. Flow (RTOR)		20			20			20	115		20		
nimed mines (a) 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Link Speeu (mpn)		512			3U 640			211			30		
Sant Base (br) Int No No No Sant Base (br) 0.94 0.94 0.90 0.92 0.	Travel Time (s)		11 7			14.5			71			9.4		
Seak Hour Each 0.94 0.94 0.94 0.90 0.90 0.92 <th0.92< th=""> 0.92 0.92</th0.92<>	Confl Bikes (#/hr)		11.7	3		14.5			7.1			7.4		
leavy Vehicles (%) 0% 3% 4% 0% 3% 0% 2% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%	Peak Hour Factor	0.94	0.94	0.94	0.90	0.90	0.90	0.92	0.92	0.92	0.92	0.92	0.92	
bas Blockages (rhn) 0 0 0 7 0 0 7 0 0 0 0 0 0 0 0 0 0 0 0	Heavy Vehicles (%)	0%	3%	4%	0%	3%	0%	2%	0%	0%	0%	0%	0%	
Parking (hhr) 0 551 78 127 666 0 70 0 115 30 28 37 Shared Lang (rolp) 0 551 78 127 666 0 70 0 115 0 95 0 Lim Type NA Perm ILD (rolp) 1 1 1 3 3 3 2 Venciced Phases 1 1 4 14 3 3 3 2 Venciced Phases 1 1 4 14 3 4 3 3 3 2 Venciced Phases 1 1 4 14 3 4 3 <	Bus Blockages (#/hr)	0	0	7	0	7	0	0	0	0	0	0	0	
big Flow (ph) 0 551 78 127 666 0 70 0 115 30 28 37 are Group Flow (ph) 0 551 78 127 666 0 70 0 115 0 95 0 Unin Type NA Perm Ph.V Ph. Ph.P. Ph.Ph.Ph.Ph.Ph.Ph.Ph.Ph.Ph.Ph.Ph.Ph.Ph.P	Parking (#/hr)							0	0	1	1	1	1	
shared Lane Iranic (%) and Four Type NA Perm DP-P NA DPm Prov Perm NA Primited (%) and Type Vehicled Phases 1 1 1 4 1 3 3 3 2 Permited Phases 1 1 1 4 14 3 4 3 3 2 Permited Phases 1 1 1 4 14 3 4 3 3 2 Permited Phases 1 1 1 4 14 3 4 3 3 2 Permited Phases 1 1 1 4 14 3 4 3 3 2 Permited Phases 1 1 1 4 14 3 4 3 3 2 Permited Phases 2 Permite	Adj. Flow (vph)	0	551	78	127	666	0	70	0	115	30	28	37	
ane store prov (pn) 0 bb 78 12 0 66 0 70 0 115 0 95 0 Volected Phases 1 1 4 14 4 14 4 3 3 2 Volected Phases 1 1 4 14 3 3 3 3 Detector Phase 1 1 4 14 3 3 3 3 Detector Phase 1 1 4 14 3 4 3 3 3 Detector Phase 1 1 4 14 3 4 3 3 Detector Phase 1 1 4 14 3 4 3 3 Detector Phase 1 1 4 14 3 4 3 3 Detector Phase 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 1 1 1 4 14 3 4 4 3 3 Detector Phase 2 1 1 1 4 14 3 4 4 3 3 Detector Phase 2 1 1 1 4 14 3 4 4 3 3 Detector Phase 2 1 1 1 4 14 3 4 4 3 4 3 3 Detector Phase 2 1 1 1 1 4 14 3 4 13 5 4 5 4 5 4 5 4 5 4 5 4 5 5 5 5 5 4 5 5 7 5 5 7 5 5 7 5 5 7 5 7	Shared Lane Traffic (%)	-		70	407		•	70	•		•	05	•	
num pec nvx Prime Prim Prime Prime <th< td=""><td>Lane Group Flow (vph)</td><td>0</td><td>551</td><td>/8</td><td>127</td><td>666</td><td>0</td><td>/0</td><td>0</td><td>115</td><td>Dorm</td><td>95</td><td>0</td><td></td></th<>	Lane Group Flow (vph)	0	551	/8	127	666	0	/0	0	115	Dorm	95	0	
concent mates 1 1 1 3 3 3 Delector Phase 1 1 4 14 3 3 3 Delector Phase 1 1 4 14 3 4 3 3 Delector Phase 1 1 4 14 3 4 3 3 Minimum Split (s) 160 160 110 144 14 3 4 3 3 Otal Split (%) 345.% 345.% 164.% 23.6% 164.% 23.6% 26.0% 28.0 28	Protected Phases		INA 1	Perm	U.P+P	NA 1.4		D.PM		pin+ov	Perm	INA 2		2
Contraction 1 1 4 1 3 3 3 Which Phase 1 1 4 14 3 4 3 3 Which Phase 1 10 14.0 11.0 14.0 11.0 14.0 14.0 14.0 28.0 Circle Split (s) 36.0 38.0 18.0 26.0 18.0 26.0 28.0 Circle Split (s) 33.0 3.0 3.0 20.0 13.0 20.0 22.00 22.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 20.0 20.0 22.0 20.0 20.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 20.0 22.0 22.0 22.0 22.0	Pormitted Phases		1	1	4	14		2		4	2	3		2
Animum Shift Phase 1 2 1 <th1< th=""> 1 <th1< th=""></th1<></th1<>	Detector Phases		1	1	4	14		3 2		3	с 2	3		
Alinnum Spit (s) 10.0 10.0 6.0 8.0 6.0 8.0 8.0 4.0 Alinnum Spit (s) 16.0 11.0 14.0 11.0 14.0 <	Switch Phase				T	14		J		Ŧ	5	5		
Attimum Split (s) 16.0 10.0 14.0 11.0 14.0 1	Minimum Initial (s)		10.0	10.0	6.0			8.0		6.0	8.0	8.0		4.0
Total Split (%) 34.0 38.0 18.0 26.0 18.0 26.0 28.0 28.6% 22.6%	Minimum Split (s)		16.0	16.0	11.0			14.0		11.0	14.0	14.0		28.0
Total Split (%) 34 5% 34 5% 64 % 23 % 16 4% 23 6% 24 % 25 % 30 <	Total Split (s)		38.0	38.0	18.0			26.0		18.0	26.0	26.0		28.0
daamum Green (s) 32.0 32.0 32.0 13.0 20.0 13.0 20.0 24.0 (ellow Time (s) 3.0 </td <td>Total Split (%)</td> <td></td> <td>34.5%</td> <td>34.5%</td> <td>16.4%</td> <td></td> <td></td> <td>23.6%</td> <td></td> <td>16.4%</td> <td>23.6%</td> <td>23.6%</td> <td></td> <td>25%</td>	Total Split (%)		34.5%	34.5%	16.4%			23.6%		16.4%	23.6%	23.6%		25%
lethov inne (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0	Maximum Green (s)		32.0	32.0	13.0			20.0		13.0	20.0	20.0		24.0
wi-red rune (s) 3.0 3.0 2.0 3.0 2.0 3.0 1.0 os Time A(s) (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 fold Los Time A(s) (s) 0.0 0.0 0.0 0.0 0.0 colar Los Time A(s) (s) 0.0 0.0 0.0 0.0 0.0 colar Los Time A(s) (s) 0.0 0.0 0.0 0.0 0.0 colar Los Time A(s) (s) 0.0 0.0 0.0 0.0 0.0 0.0 colar Los Time A(s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	Yellow Time (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0
Las I mine August (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	All-Red Time (s)		3.0	3.0	2.0			3.0		2.0	3.0	3.0		1.0
Usin List (1111 (c)) 0,0 0,0 0,0 0,0 0,0 0,0 0,0 0,0 0,0 0	Lost Lime Adjust (s)		0.0	0.0	0.0			0.0		0.0		0.0		
Lean Lean Lean Lean Lean Lean Lean Lean	Load/Lag		0.0	0.0	5.0			0.0		5.0	Load	0.0		Log
Local of purple Los Los <thlos< th=""></thlos<>	Lead-Lag Ontimize?		Ves	Ves	Ves			Yes		Ves	Yes	Yes		Yes
Accal Mode C.Max C.Max C.Max None None<	Vehicle Extension (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0
Walk Time (s) 1.000 1000 1000 1000 1000 1000 1000 17.0 Tash Dont Walk (s) 17.0 17.0 18.0 1000 11.0 1000 11.0 1000 1000 11.0 1000	Recall Mode		C-Max	C-Max	None			None		None	None	None		None
Flash Dont Walk (s) 17.0 Pedestrian Calls (#hr) 94 Vect Elf CI Green (s) 41.3 41.3 58.6 62.6 13.0 95.3 13.0 94 Vectatio Q) C Ratio 0.38 0.38 0.53 0.57 0.12 0.32 0.12 13 12 12 13 13 12 13	Walk Time (s)				12.10							0.10		7.0
Pedestrian Calls (#hr) 58.6 62.6 13.0 35.3 13.0 Act Effct Green (s) 41.3 58.6 62.6 13.0 35.3 13.0 Act Effct Green (s) 0.38 0.38 0.53 0.57 0.12 0.32 0.57 Vic Ratio 0.47 0.16 0.26 0.38 0.59 0.23 0.57 Control Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 Joueue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 OS C C B E A E December 10 58.1 Outeue Length 50th (1) 175 41 44 144 48 0 65 Duceue Length 95th (1) 123 81 87 231 332 December 10 December 10 120 125 80 130 130 14 144 48 0 0 0 0 0 0 0 0 130 131 131 130 130 130 130	Flash Dont Walk (s)													17.0
kdt Effer Green (s) 41.3 41.3 58.6 62.6 13.0 35.3 13.0 kctuated g/C Ratio 0.38 0.38 0.53 0.57 0.12 0.32 0.12 ic Ratio 0.47 0.16 0.26 0.38 0.59 0.23 0.57 Control Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 Dueue Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 Dueue Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 DOS C C B B B E A E Approach Delay 31.1 16.1 58.1 Approach LOS C B B B E A E Dueue Length 50th (ft) 175 41 44 144 48 0 65 Dueue Length 50th (ft) 175 41 44 144 48 0 65 Dueue Length 50th (ft) 175 41 44 144 48 0 65 Dueue Length 50th (ft) 175 41 44 144 48 0 65 Dueue Length 50th (ft) 175 41 74 144 144 48 0 65 Dueue Length 50th (ft) 175 12 125 80 Sase Capacity (vph) 1183 497 490 1753 184 502 259 Starvation Cap Reductin 0 0 0 0 0 0 0 0 0 0 Spillback Cap Reductin 0 0 0 0 0 0 0 0 0 Reduced v/c Ratio 0.47 0.16 0.26 0.38 0.38 0.23 0.37 ThereseClion Summary Vea Type: CBD Syde Length 110 Starvation Cap Referenced to phase 1:EBWB, Start of Green Vatural Cycle Length 10 Starvation Signal Delay: 25.3 Intersection LOS: C Intersection Signal Delay: 25.3 Intersection LOS: C Spills and Phases: 3. Willow Street & Centre Streel	Pedestrian Calls (#/hr)													84
Actuated g/C Ratio 0.38 0.38 0.38 0.53 0.57 0.12 0.32 0.12 //c Ratio 0.47 0.16 0.26 0.38 0.59 0.23 0.57 0.57 0.57 0.5 0.58 58.1 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Act Effct Green (s)		41.3	41.3	58.6	62.6		13.0		35.3		13.0		
<i>ic</i> Ratio 0.47 0.16 0.26 0.38 0.59 0.23 0.57 Control Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 Dueue Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 OS C C B E A E Approach LOS C C B E A E Dueue Length 50h (ft) 175 41 44 144 48 0 65 Dueue Length 50h (ft) 234 81 87 213 91 38 113 Itrum Bay Length (ft) 1234 81 87 213 32 32 Stase Capacity (reph) 1183 497 490 1753 184 502 259 Starvation Cap Reductn 0 0 0 0 0 0 0 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Actuated g/C Ratio		0.38	0.38	0.53	0.57		0.12		0.32		0.12		
John To Delay J. J. 29, 6 15, 4 16, 2 64, 5 5, 8 58, 1 Joueue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 31.3 29, 6 15, 4 16, 2 64, 5 5, 8 58, 1 JOS C C B B E A E Approach Delay 31.1 16, 1 58, 1 58, 1 58, 1 Jueue Length Sth (ft) 175 41 44 144 48 0 65 Dueue Length Sth (ft) 234 81 87 213 91 38 113 Turn Bay Length (ft) 120 125 80 322 259 Savariator Cap Reductin 0 0 0 0 0 0 Spillback Cap Reductin 0 0 0 0 0 0 0 Stavatior Cap Reductin 0 0 0 0 0 0 0 0 0 Stavatior Cap Reductin 0.47 0.16 0.26	v/c Ratio		0.47	0.16	0.26	0.38		0.59		0.23		0.57		
Debug beday 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Join Delay 31.3 29.6 15.4 16.2 64.5 5.8 58.1 LOS C C B B E A E Approach Delay 31.1 16.1 58.1 S8.1 Approach LOS C B E C E Dueue Length 50th (ft) 175 41 44 144 48 0 65 Dueue Length 95th (ft) 234 81 87 213 91 38 113 nternal Link Dist (ft) 172 120 125 80 332 Juneue Length 95th (ft) 1183 497 490 1753 184 502 259 Starvation Cap Reductin 0 0 0 0 0 0 0 Storage Cap Reductin 0 0 0 0 0 0 0 Storage Cap Reductin 0 0 0 0 0 0 0 Storage Cap Reductin 0 0 0 0 0 0 0 Vice Length: 110 U U U U U <td>Control Delay</td> <td></td> <td>31.3</td> <td>29.6</td> <td>15.4</td> <td>16.2</td> <td></td> <td>64.5</td> <td></td> <td>5.8</td> <td></td> <td>58.1</td> <td></td> <td></td>	Control Delay		31.3	29.6	15.4	16.2		64.5		5.8		58.1		
Orac Cecky 31.3 27.0 10.4 10.4 04.3 3.6 36.1 QS C C B E A E Approach LOS C B E A E Duce Length S0th (ft) 175 41 44 144 48 0 65 Duce Length 95th (ft) 234 81 87 213 91 38 113 Internal Link Dist (ft) 433 560 231 332 332 furn Bay Length (ft) 1183 497 490 1753 184 502 259 Starvation Cap Reductn 0	Total Delay		0.0	0.0	0.0	0.0		0.0		0.0		0.0		
Corr D D C A E Approach Delay 31.1 16.1 58.1 Approach LOS C B E Dueue Length S0th (ft) 175 41 44 144 48 0 65 Dueue Length S0th (ft) 234 81 87 213 91 38 113 Internal Link Dist (ft) 433 560 231 332 332 furn Bay Length (ft) 120 125 80 338 1332 Sase Capacity (wph) 1183 497 490 1753 184 502 259 Starvation Cap Reductn 0 0 0 0 0 0 0 SpliBack Cap Reductn 0 0 0 0 0 0 0 Starge Cap Reductn 0 0 0 0 0 0 0 0 Starge Cap Reductn 0 0.26 0.38 0.38 0.23 0.37 Veatureet Citon Summary Verea Type: CBD CBD Verea Ty			31.3	29.0	10.4 D	10.2 D		04.5 E		ö.C		00.1 E		
the second s	Approach Delay		31.1	- C	D	16.1		E		A		58.1		
Dueue Length 50th (ft) 175 41 44 144 48 0 65 Dueue Length 95th (ft) 234 81 87 213 91 38 113 Internal Link Dist (ft) 433 560 231 332 113 Internal Link Dist (ft) 120 125 80 Base Capacity (vph) 1183 497 490 1753 184 502 259 Starvation Cap Reductin 0	Approach LOS		51.1 C			B						F		
Dueue Length 95th (ft) 234 81 87 213 91 38 113 nternal Link Dist (ft) 433 560 231 332 332 furm Bay Length (ft) 120 125 80 338 332 sase Capacity (vph) 1183 497 490 1753 184 502 259 Slarvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Otage Cap Reductn 0.47 0.16 0.26 0.38 0.38 0.23 0.37	Queue Length 50th (ft)		175	41	44	144		48		0		65		
nternal Link Dist (ft) 433 560 231 332 Turn Bay Length (ft) 120 125 80 Sase Capacity (vph) 1183 497 490 1753 184 502 259 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 Storage Cap Reductn 0 0.16 0.26 0.38 0.38 0.23 0.37 Intersection Summary Vera Type: CBD Sycel Length: 110 Cycle Length: 110 Storate C Ordinated Aaximum vic Ratio: 0.59 Intersection Capacity Utilization 49.3% ICU Level of Service A Intersection Capacity Utilization 49.3% ICU Level of Service A Intersection Capacity Utilization 49.3% ICU Level of Service A Splits and Phases: 3: Willow Street & Centre Street	Queue Length 95th (ft)		234	81	87	213		91		38		113		
furn Bay Length (ft) 120 125 80 Jase Capacity (vph) 1183 497 490 1753 184 502 259 Starvation Cap Reductn 0 0 0 0 0 0 Starvation Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0.47 0.16 0.26 0.38 0.38 0.23 0.37	Internal Link Dist (ft)		433			560			231			332		
Base Capacity (vph) 1183 497 490 1753 184 502 259 Starvation Cap Reductin 0 0 0 0 0 0 0 Sillback Cap Reductin 0 0 0 0 0 0 0 Sillback Cap Reductin 0 0 0 0 0 0 0 Sillback Cap Reductin 0 0 0 0 0 0 0 Sillback Cap Reductin 0 0.47 0.16 0.26 0.38 0.38 0.23 0.37 Intersection Summary Vector Signal Pelay 25.3 Intersection LOS: C Notice Length: 110 Offset: 0(0%), Referenced to phase 1:EBWB, Start of Green Vatural Cycle: 70 200 25.3 Intersection LOS: C Intersection LOS: C Intersection Signal Delay: 25.3 Intersection LOS: C Intersection Grapicly Uillization 49.3% ICU Level of Service A Nalysis Period (min) 15 5 5 5 5	Turn Bay Length (ft)			120	125					80				
Starvation Cap Reductn 0 <td>Base Capacity (vph)</td> <td></td> <td>1183</td> <td>497</td> <td>490</td> <td>1753</td> <td></td> <td>184</td> <td></td> <td>502</td> <td></td> <td>259</td> <td></td> <td></td>	Base Capacity (vph)		1183	497	490	1753		184		502		259		
Spillback Cap Reductn 0	Starvation Cap Reductn		0	0	0	0		0		0		0		
Storage Cap Reductn 0 0 0 0 0 0 0 Reduced v/c Ratio 0.47 0.16 0.26 0.38 0.38 0.23 0.37 Intersection Summary	Spillback Cap Reductn		0	0	0	0		0		0		0		
Vecauced vic Ratio 0.47 0.16 0.26 0.38 0.38 0.23 0.37 Intersection Summary Veca Type: CBD CBD </td <td>Storage Cap Reductn</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td></td> <td>0</td> <td></td> <td>0</td> <td></td> <td>0</td> <td></td> <td></td>	Storage Cap Reductn		0	0	0	0		0		0		0		
ntersection Summary Viea Type: CBD Cycle Length: 110 Cycle Length: 110 Offset: 0 (0%), Referenced to phase 1:EBWB, Start of Green Vatural Cycle: 70 Ontrol Type: Actuated-Coordinated Jaximum v/c Ratio: 0.59 Intersection Capacity Utilization 49.3% ICU Level of Service A Intersection	Reduced v/c Ratio		0.47	0.16	0.26	0.38		0.38		0.23		0.37		
Area Type: CBD Cycle Length: 110 Sycle Length: 110 Jiffset: 0 (0%), Referenced to phase 1:EBWB, Start of Green Valural Cycle: 70 Control Type: Actuated-Coordinated Maximum vic Ratio: 0:59 Intersection Signal Delay: 25.3 Intersection LOS: C Intersection Capacity Utilization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Intersection Summary													
Cycle Length: 110 Cycluated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 1:EBWB, Start of Green Valural Cycle: 70 Control Type: Actuated-Coordinated Aaximum vic Ratio: 0.59 ntersection Signal Delay: 25.3 Intersection LOS: C Intersection Capacity Utilization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Area Type:	CBD												
Actuated Cycle Length: 110 Diffset: 0 (0%), Referenced to phase 1:EBWB, Start of Green Vatural Cycle: 70 Control Type: Actuated-Coordinated Vaximum v/c Ratio: 0.59 ntersection Cost C Intersection LOS: C Intersection LOS: C Intersection Capacity Utilization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Cycle Length: 110													
Diffset: 0(0%), Referenced to phase 1:EBWB, Start of Green Vatural Cycle: 70 Dontrol Type: Actuated-Coordinated Maximum vic Ratio: 0.59 Intersection Capacity Utilization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Actuated Cycle Length: 110													
Vatural Cycle: 70 Control Type: Actuated-Coordinated Aaximum v/c Ratio: 0.59 Intersection Signal Delay: 25.3 Intersection LOS: C Intersection Capacity Utilization 49.3% ICU Level of Service A Inalysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Offset: 0 (0%), Referenced to	o phase 1:EB	WB, Start	of Green										
Aaximum vic Ratio: 0.59 Aaximum vic Ratio: 0.59 Intersection LOS: C Intersection Capacity Utilization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Natural Cycle: 70													
Makini Wir Kalub U. 397 Intersection Signal Delay: 25.3 Intersection LOS: C Intersection Capacity Utilization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Control Type: Actuated-Coor	rdinated												
Intersection Capacity Littlization 49.3% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Intersection Signal Delay 25	2			اسل	torcoction	108.0							
Analysis Period (min) 15 Splits and Phases: 3: Willow Street & Centre Street	Intersection Canacity Hitlizat	ion 49 2%			IN		LUS: U	1						
Splits and Phases: 3: Willow Street & Centre Street	Analysis Period (min) 15	1011 T 7.370			IC.	C LEVEI U	JUNICE F	•						
Splits and Phases: 3: Willow Street & Centre Street														
	Splits and Phases: 3: Willo	ow Street & C	Centre Stre	et										
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Lane Group	FBI	FRT	FRP	WRI	W/RT	WRP	NRI	NBT	NRP	SBI	SRT	SBD	രി	_	
Lane Configurations	LDĹ	**	7	WDL K	**	WDI	NDL	NDT	NDIN 1	JDL	<u></u>	JUN	102		
Volume (vph)	0	646	43	66	485	0	39	0	55	24	11	34			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Storage Length (ft)	0		120	125		0	0		80	0		0			
Storage Lanes	0		1	1		0	1		1	0		0			
Laper Length (II)	25	0.95	1.00	1 00	0.95	1.00	1.00	1.00	1.00	25	1.00	1.00			
Ped Bike Factor	1.00	0.75	0.98	1.00	0.75	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Frt			0.850						0.850		0.933				
Flt Protected				0.950			0.950				0.983				
Satd. Flow (prot)	0	3110	1358	1624	3080	0	1392	0	1275	0	1383	0			
Flt Permitted				0.289			0.753				0.983				
Satd. Flow (perm)	0	3110	1325	494	3080	0	1104	0	1275	0	1383	0			
Right Turn on Red			No			No			Yes			No			
Satd. Flow (RTOR)		20			20			20	/6		20				
Link Speed (mpn)		3U E12			30			211			30				
Travel Time (s)		11 7			14.5			71			41Z Q /				
Confl Bikes (#/hr)		11.7	4		14.5			7.1			7.4				
Peak Hour Factor	0.90	0.90	0.90	0.95	0.95	0.95	0.88	0.88	0.88	0.86	0.86	0.86			
Heavy Vehicles (%)	0%	3%	7%	0%	4%	0%	5%	0%	2%	0%	0%	3%			
Bus Blockages (#/hr)	0	7	0	0	7	0	0	0	0	0	0	0			
Parking (#/hr)							0	0	1	1	1	1			
Adj. Flow (vph)	0	718	48	69	511	0	44	0	62	28	13	40			
Shared Lane Traffic (%)															
Lane Group Flow (vph)	0	718	48	69	511	0	44	0	62	0	81	0			
Turn Type		NA	Perm	D.P+P	NA		D.Pm		pm+ov	Perm	NA				
Protected Phases		1		4	14				4		3		2		
Permitted Phases		1	1	1	1.4		3		3	3	2				
Delector Phase Switch Phase		1	1	4	14		3		4	3	3				
Switch Phase Minimum Initial (c)		10.0	10.0	6.0			20		60	0.0	20		4.0		
Minimum Snlit (s)		16.0	10.0	0.0			0.U 1/L0		0.0	0.U 1/L0	0.U 1/1 0		28.0		
Total Split (s)		39.0	39.0	11.0			22.0		11.0	22.0	22.0		28.0		
Total Split (%)		39.0%	39.0%	11.0%			22.0%		11.0%	22.0%	22.0%		28%		
Maximum Green (s)		33.0	33.0	6.0			16.0		6.0	16.0	16.0		24.0		
Yellow Time (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0		
All-Red Time (s)		3.0	3.0	2.0			3.0		2.0	3.0	3.0		1.0		
Lost Time Adjust (s)		0.0	0.0	0.0			0.0		0.0		0.0				
Total Lost Time (s)		6.0	6.0	5.0			6.0		5.0		6.0				
Lead/Lag		Lead	Lead	Lag			Lead		Lag	Lead	Lead		Lag		
Lead-Lag Optimize?		Yes	Yes	Yes			Yes		Yes	Yes	Yes		Yes		
Vehicle Extension (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0		
Recall Mode		C-Max	C-Max	None			None		None	None	None		None 7.0		
Walk Time (S)													17.0		
Pedestrian Calls (#/br)													61		
Act Effet Green (s)		43.0	43.0	52.9	58.1		11.5		23.6		11 5		01		
Actuated d/C Ratio		0.43	-13.0	0.53	0.58		0.12		0.24		0.12				
v/c Ratio		0.54	0.08	0.19	0.29		0.35		0.17		0.51				
Control Delay		27.6	24.1	15.8	15.3		47.3		6.1		52.2				
Queue Delay		0.0	0.0	0.0	0.0		0.0		0.0		0.0				
Total Delay		27.6	24.1	15.8	15.3		47.3		6.1		52.2				
LOS		С	С	В	В		D		А		D				
Approach Delay		27.4			15.4						52.2				
Approach LOS		С			В						D				
Queue Length 50th (ft)		208	21	22	101		26		0		49				
Queue Length 95th (ft)		275	48	51	154		57		22		88				
Internal Link Dist (ft)		433			560			231			332				
Turn Bay Length (ft)		100/	120	125	1700		17/		80		004				
Base Capacity (vph)		1336	569	362	1/89		1/6		359		221				
Starvation Cap Reductn		0	0	0	0		0		0		0				
Spinuaux Cap Reductin		0	0	0	0		0		0		0				
Reduced v/c Ratio		0.54	0 08	0 10	0 20		0 25		0 17		0 37				
		0.04	0.08	U.17	0.29		0.20		U.17		0.37				
Intersection Summary															
Area Type:	CBD														
Cycle Length: 100															
Actuated Cycle Length: 100	the sets of the														
Uffset: 80 (80%), Referenced	to phase 1:	EBWB, Sta	art of Gree	n											
Natural Cycle: /5	dipotod														
Control Type: Actuated-Coord Maximum v/c Patio: 0.54	unated														
Intersection Signal Delay: 22	0			In	arcaction	105.0									
Intersection Canacity Utilization	on 50.1%				U evel of	Service 4									
Analysis Period (min) 15	011 00.170			ic	C LOVEI UI	JUNICE F									
inarysis i chod (min) 15															
Splits and Phases: 3: Willo	w Street & C	Centre Stre	et												
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ane Group	FBI	FRT	FRR	WBI	WRT	WRR	NRI	NRT	NBR	SBI	SBT	SBR	ø2
ane Configurations	LDL	*	LDK 7	WDL	**	WDR	NDL	NDT	NDK 7	JDL	301 4	JUK	WZ.
Volume (vph)	0	518	73	114	602	0	64	0	106	30	26	36	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0		120	125		0	0		80	0		0	
Storage Lanes	25		1	30		0	100		1	25		0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor			0.98										
Frt			0.850						0.850		0.947		
Flt Protected				0.950		_	0.950				0.984	_	
Satd. Flow (prot)	0	3154	1358	1624	3110	0	1433	0	1301	0	1426	0	
Satd Flow (perm)	0	3154	1326	624	3110	0	996	0	1301	0	1426	0	
Right Turn on Red	Ū	0101	No	021	0110	No	770		Yes		1120	No	
Satd. Flow (RTOR)									115				
Link Speed (mph)		30			30			30			30		
Link Distance (ft)		513			640			311			412		
Travel Time (s)		11./	2		14.5			7.1			9.4		
Conii. Bikes (#/nr) Peak Hour Factor	0.04	0.04	0.04	0.00	0 00	0.00	0.02	0.02	0.02	0 02	0.02	0.02	
Heavy Vehicles (%)	0%	3%	4%	0.70	3%	0%	2%	0.72	0.72	0.72	0.72	0.72	
Bus Blockages (#/hr)	0	0	7	0	7	0	0	0	0	0	0	0	
Parking (#/hr)							0	0	1	1	1	1	
Adj. Flow (vph)	0	551	78	127	669	0	70	0	115	33	28	39	
Shared Lane Traffic (%)			76	407	(()	•	70	•		~	100	•	
Lane Group Flow (vph)	0	551	/8 Dorm	127 D P P	669 MA	0	70 D.Pm	0	115 pm: ov	Dorm	100	0	
Protected Phases		1	renn	U.P+P 4	1.4		U.P111		μπ+0v 4	Felli	3		2
Permitted Phases			1	1	14		3		3	3	5		2
Detector Phase		1	1	4	14		3		4	3	3		
Switch Phase													
Minimum Initial (s)		10.0	10.0	6.0			8.0		6.0	8.0	8.0		4.0
Minimum Split (s)		16.0	16.0	11.0			14.0		11.0	14.0	14.0		28.0
Total Split (S) Total Split (%)		38.0	38.0	18.0			26.0		16 /0/	26.0	26.0		28.0
Maximum Green (s)		34.5%	34.5%	10.4%			∠3.0% 20.0		10.4%	∠3.0% 20.0	23.0% 20.0		∠3% 24.0
Yellow Time (s)		3.0	3.0	3.0			3.0		3.0	3.0	3.0		3.0
All-Red Time (s)		3.0	3.0	2.0			3.0		2.0	3.0	3.0		1.0
Lost Time Adjust (s)		0.0	0.0	0.0			0.0		0.0		0.0		
Total Lost Time (s)		6.0	6.0	5.0			6.0		5.0		6.0		
Lead/Lag		Lead	Lead	Lag			Lead		Lag	Lead	Lead		Lag
Lead-Lag Optimize?		Yes	Yes	Yes			Yes		Yes	Yes	Yes		Yes
Pecall Mode		C-Max	C-Max	None			S.U None		None	None	None		None
Walk Time (s)		C-IVIUX	C-IVIDA	NULL			NULL		NOTIC	NONC	NONC		7.0
Flash Dont Walk (s)													17.0
Pedestrian Calls (#/hr)													84
Act Effct Green (s)		41.2	41.2	58.4	62.4		13.2		35.4		13.2		
Actuated g/C Ratio		0.37	0.37	0.53	0.57		0.12		0.32		0.12		
V/c Ratio		0.47	0.16	0.27	0.38		0.59		0.23		0.59		
		31.3	29.7	10.0	10.4		04.0		0.0		0.0		
Total Delay		31.3	29.7	15.6	16.4		64.6		5.8		58.8		
LOS		С	С	В	В		E		A		E		
Approach Delay		31.1			16.3						58.8		
Approach LOS		С			В						E		
Queue Length 50th (ft)		175	41	44	146		48		0		68		
Queue Length 95th (tt)		234	81	8/	215		92	221	38		118		
Turn Bay Length (ft)		433	120	125	000			231	80		332		
Base Capacity (vnh)		1180	496	487	1747		181		503		259		
Starvation Cap Reductn		0	0	0	0		0		0		0		
Spillback Cap Reductn		0	0	0	0		0		0		0		
Storage Cap Reductn		0	0	0	0		0		0		0		
Reduced v/c Ratio		0.47	0.16	0.26	0.38		0.39		0.23		0.39		
Intersection Summary													
Area Type:	CBD												
Cycle Length: 110													
Actuated Cycle Length: 110													
Offset: 0 (0%), Referenced t	o phase 1:EB	WB, Start	of Green										
Natural Cycle: /0 Control Type: Actuated Coord	rdinated												
Maximum v/c Ratio: 0.59	UIIIdleU												
Intersection Signal Delay: 25	5.5			In	tersection	LOS: C							
Intersection Capacity Utilizat	tion 49.6%			IC	U Level o	f Service /	Ą						
Analysis Period (min) 15													
o. III I													
Splits and Phases: 3: Will	ow Street & C	entre Stre	et			1							
= a1 (P)						1						1.	